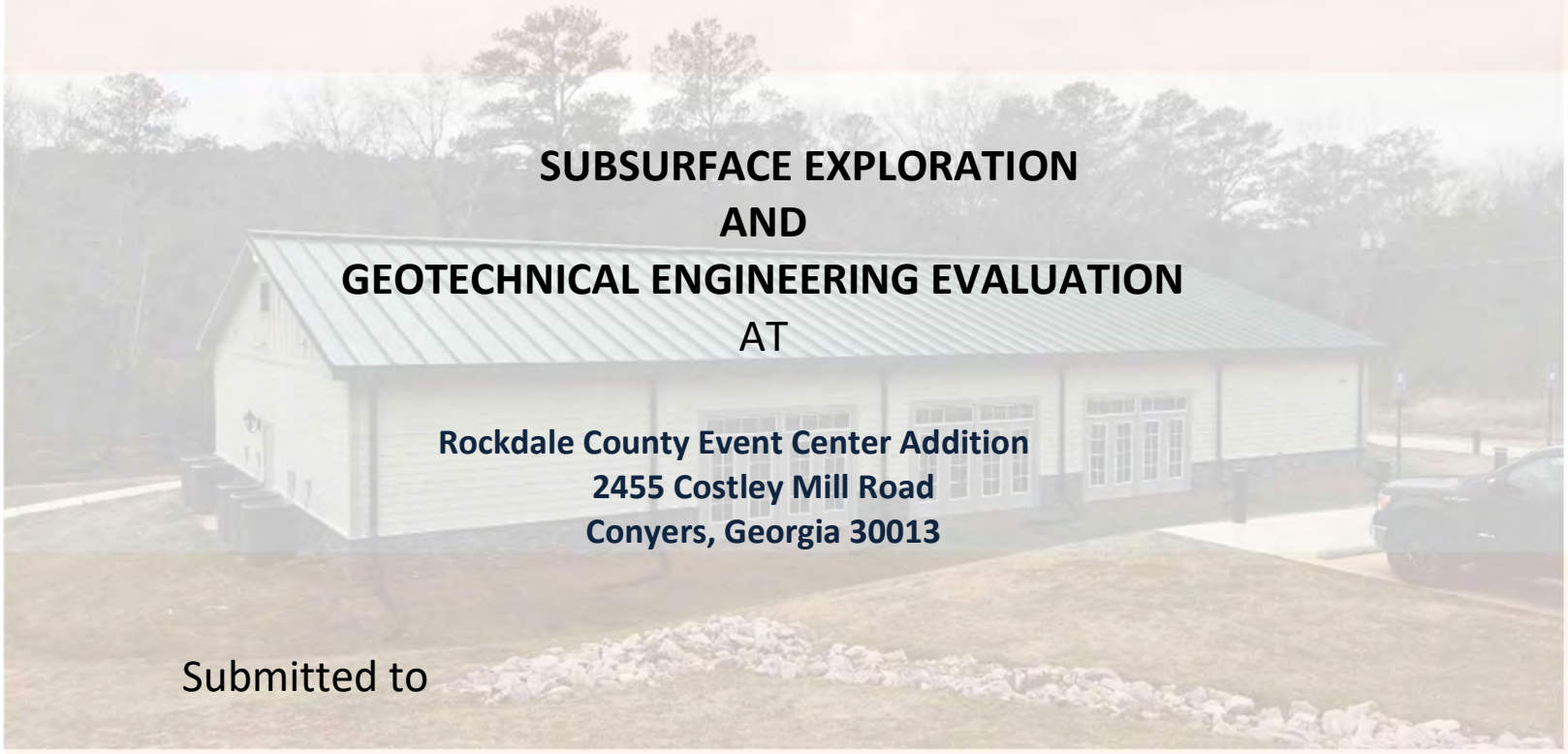
Exhibit #4



SUBSURFACE EXPLORATION
AND
GEOTECHNICAL ENGINEERING EVALUATION
AT

Rockdale County Event Center Addition
2455 Costley Mill Road
Conyers, Georgia 30013

Submitted to

Mr. Michael Robinson
Senior Capital Projects Manager
Department of General Services
Rockdale County PO Box 289
cConyers, GA 30012

January 2024

MEG 302287.61

MATRIX ENGINEERING GROUP
25 YEARS OF ENGINEERING QUALITY
Est. 1994
engineers | special inspectors | construction consultants

January 31, 2024

Mr. Michael Robinson
Senior Capital Projects Manager
Department of General Services
Rockdale County PO Box 289
Conyers, GA 30012

Re: Subsurface Exploration and Geotechnical Engineering Evaluation
Rockdale County Event Center Addition- 2599 Salem Church Road, Conyers, GA 30013
Matrix Engineering Group Project Number MEG-302287.61

Dear Mr. Robinson:

Matrix Engineering Group, Inc. has completed the geotechnical exploration and evaluation for the proposed Rockdale County Event Center Addition project located at 2599 Salem Church Road in Conyers, Georgia 30013.

The scope of this work included drilling a total of five (5) soil test borings within the area planned for the development and providing the findings and recommendations regarding the geotechnical aspects of the proposed project.

This work was performed in general accordance with Matrix Proposal Number 121923-1, dated December 19, 2023, and the subsequent authorization to proceed by Mr. Marcello Banes on January 3rd, 2024.

Introduction

The site is located at 2599 Salem Church Road in Conyers, Georgia 30013. It is at the southeastern end of the existing Event Center Building. The ground cover at the site is vegetation and topsoil. We noted rock outcrops and pile of rocks in the vicinity of the site.

Based on our site visit and the Grading Plan dated 4/02/2018, the site slopes down gently in an easterly direction from the southeastern end of the existing building at an approximate elevation of 679 feet Mean Sea Level (MSL) to a Grass Swale at an approximate elevation of 676 feet MSL.

We understand that a 10’ x 50’ addition is being proposed at the southeastern end of the existing Event Center Building. According to the Grading Plan dated 4/2/2018, the Finished Floor Elevation of the existing building is 680 feet MSL.

Exploration and Testing Program

The geotechnical exploration program consisted of the drilling and sampling of a total of five (5) soil test borings within the proposed addition area. The approximate locations of the soil test borings are shown on Figure 1 presented in the Appendix of this report. For exact locations, the owner may elect to survey the boring locations. Matrix should be informed of any deviations in order to evaluate and modify our recommendations, if necessary.

The test borings were performed utilizing a track mounted with CME 75 drill rig equipped with an automatic hammer in general accordance with ASTM D1586 standards. The planned depths of the borings ranged between 10 feet to 20 feet BGS. Borings were advanced by auguring through the soils with continuous flights of 3 $^{1}/_{4}$-inch ID augers. At regular intervals, soil samples were obtained through the center of the auger flights with a standard 1.4-inch I.D., 2-inch O.D., split-tube sampler. The sampler is first seated 6 inches to penetrate loosened strata before sampling, and then driven an additional foot with blows of a 140-pound hammer falling 30 inches. The number of hammer blows required to drive the sampler the final foot is recorded and is designated as the Standard Penetration Resistance (N-Value). The penetration resistance, when properly evaluated, is an index of the soil strength, consistency and ability to support foundations.

Representative soil samples were obtained using split-spoon sampling techniques. The samples were classified in the field in general accordance with ASTM D2488 (Visual-Manual Procedure for Description of Soils). Representative portions of the soil samples were placed in sealable, plastic bags and transported to our laboratory. During the field operations, Matrix staff maintained a continuous log of the subsurface conditions including changes in the stratigraphy and any observed groundwater levels. Soil descriptions and penetration resistance values are presented graphically on the Soil Boring Records included in the Appendix of this report.

All borings were backfilled with the soil cuttings by Matrix Engineering staff at the conclusion of the field work. Some consolidation of the backfilled soil column should be expected over time.

Subsurface Conditions

The subsurface conditions were characterized by visual-manual examination of the soils obtained from the split-spoon sampler and observation from the auger cutting during the drilling and auguring operations. The soil boring logs, designated as B-1 to B-5, are provided in the Appendix of this report.

The subsurface conditions within the drilled borings are characterized as follows:

Man-made fill was encountered within the top 1 foot to 4.5 feet below the existing ground surface (BGS). The man-made fill consisted of Loose, wet, Silty Sand (SM) with hairline roots and gravel. The standard penetration resistance within the fill was 6 blows per foot (bpf).

Residual soils are those which have weathered in place from the parent rock. Residual soils were encountered beneath the fill and consisted of Medium Dense Silty Sand (SM). The standard penetration resistance within the residual soils ranged between 17 bpf and 18 bpf.

Partially Weathered Rock (PWR) is a regionally used term for residual material with a Standard Penetration Resistance (N-values) of 100 bpf or more, but which can be penetrated by the soil drilling equipment. PWR was encountered at borings B-1, B-2, and B-3 at depths ranging from 2 feet to 4.5 feet BGS. Auger refusal was encountered at all borings at depths ranging from 1 foot to 6 feet BGS. Rock coring was not included in our scope.

Groundwater was not encountered within the drilled depths at the time of drilling. Groundwater elevations do fluctuate with seasonal changes and typically vary on the order of 4 to 8 feet. Refer to the Appendix of this report for the boring logs and cross-section plan for the site.

Recommendations:

The following recommendations are based on the information furnished to us, the data obtained from the subsurface exploration, and our past experience with similar projects. They were prepared in general accordance with established and accepted professional geotechnical engineering practice in this region. Our recommendations are based on findings from the dates referenced within this report and do not reflect any variations that would likely exist at later dates or between the pre-designated borings or unexplored areas.

If information becomes available which may impact our recommendations, Matrix Engineering Group shall be afforded the opportunity to review this information and re-evaluate the recommendations contained within this report and make any alterations deemed necessary by a Georgia Registered professional engineer. This report is intended for the use of Rockdale County and its current design team. No other warranty is expressed or implied. Matrix Engineering Group, Inc. is not responsible for conclusions, opinions, or recommendations made by others based on this report.

The following recommendations present general guidelines for the proposed development:

Subgrade Preparation

The test borings indicated that, the top 1 foot to 4.5 feet of the soil at the site consisted of loose, and wet man-made fill containing hairline roots and gravel. The subgrade preparation for the proposed addition should be the stripping of vegetation and topsoil, the removal of the top few feet of soil, and any deleterious materials, if encountered. Topsoil can be used in landscape areas. Any other items, such as underground utility lines, or trash pits that may be encountered during the grading operation should be treated on an individual basis and brought to the attention of the Geotechnical Engineer for evaluation and recommendations.

After removal of the surface materials, the suitability of the exposed subgrade should be confirmed by proofrolling, which will discern any localized soft zones in the subgrade. The proofrolling should be performed by a loaded tandem-wheeled dump truck with an approximate weight of 25 tons. Any material that deflects excessively or ruts under the loaded truck should be densified or removed and replaced with well-compacted material.

Similarly, the suitability of all other areas of the exposed subgrade needs to be confirmed by proofrolling at the time of construction, after any unsuitable or softened materials are removed. The proofrolling should be observed by the geotechnical engineer. Structural fill procedures are provided in Section 8.1 of this report.

Slab-On-Grade Construction

The concrete slab-on-grade for the proposed addition should be supported on compacted, and properly prepared, soil subgrade. Provided the fill material and/or existing subgrade is installed to a minimum of 98% of the Standard Proctor's maximum dry density, a modulus of subgrade reaction (k) of 125 pci can be used for designing the floor slab-on-grade. Slab reinforcement and joint spacing should be carefully considered to control random cracking due to slab shrinkage. We recommend that a 10-mil vapor barrier/retarder (such as polyethylene) be installed below the (slab-on-grade) concrete to limit intrusion of water vapor through the slab. Beneath slab-on-grade areas, a minimum of 4 inches of clean, densely graded, granular material with a balanced content of fines is recommended to facilitate fine grading and provide stable surface for construction traffic and building loads. **Open-graded bases (such as #57 stone) do not meet these requirements because they are relatively incompatible, difficult to trim, and are unstable for construction traffic.** It is also difficult to fine grade an open-graded base to a relatively uniform elevation, which can result in restraint to concrete movement as the concrete cools or dries, thus increasing the probability of out-of-joint cracking. If open-graded bases are specified, the

surface of these bases should be choked off with a clean fine-graded material with at least 10 to 30% of the particles passing a No. 100 sieve, but not contaminated with clay, silt, or organic material.

Foundation

Based on the testing performed, the site appears to be suitable for construction of the proposed addition provided the subgrade preparation is performed in accordance with our recommendations. We recommend that a maximum allowable soil bearing capacity of **2,500 psf** be used to design the foundations. We recommend that after the foundations are excavated, the bottom of the footings should be inspected by the County or a third-party inspection agency in order to verify the recommended bearing capacity and to delineate any soft areas, if encountered.

We recommend that all **continuous footings have a minimum width of 2 feet and should be a minimum 18 inches** below subgrade elevations to prevent shear failure and to minimize the effects of frost.

Total settlement of footing foundations is estimated to be on the order of 1 inch or less. Based on the subsurface conditions and our experience with similar soils, we do not anticipate differential settlements to exceed $^1/_2$ inch between column supports (assuming 50 foot spacing or less)

Structural Fill Procedure

Staged, methodical and well-planned grading is key to avoiding unnecessary costs and time delays. Areas should not be stripped or disturbed if the grading contractor is unable to properly seal the subgrade prior to departure each day. Exposure of soils to moisture from direct rainfall or runoff usually renders these soils un-usable for several days. This usually gets mischaracterized as an unsuitable soils condition which is inaccurate. Unsuitable soils are defined as those containing deleterious matter (such as organics, alluvium, debris and/or trash). Moisture related problems should be avoided by employing best management practices that involve maintaining positive drainage, installation of berms, diversion channels, and/or sealing the subgrade to avoid water infiltration. Other measures involve covering all stockpiled soils with heavy tarps or plastic to avoid saturating the soils in the event of rainfall. Means and methods of construction are certainly the contractor's jurisdiction; however, exposing otherwise suitable soils to excessive moisture or softening of existing subgrades as a result of unscrupulous construction traffic should be avoided and planned for.

We recommend that the following criteria be used for structural fill:

1. Adequate laboratory proctor density tests should be performed on representative samples of the proposed fill materials to provide data necessary for the quality control. The moisture content at the time of compaction should be within 3 percentage points of the optimum moisture content. In addition, we recommend that the fill soils be free of organic matter and rock boulder/cobbles larger than 2 inches in nominal size and relatively non-plastic with plasticity indices less than 20.
2. Suitable fill material should be placed in thin lifts (lift thickness depends on type of equipment used, but generally lifts of 8 inches loose measurements are recommended). The soil should be compacted by mechanical means such as sheepsfoot rollers.
3. Slopes that are limited to 2:1 (horizontal: vertical), or flatter, will have adequate long term slope stability, if limited in height to 15 feet, based on our experience with the type of soils encountered onsite. The slope's crest should be protected against water ponding. Proposed slopes should incorporate only suitable fill, clean of any organics or any other vegetative content. Topsoil should only be used to provide cover over the completed slope's free face so as to promote vegetative growth which in turn protects the slope's surface against scour and erosion. Slopes should be overbuilt and cut back to the proposed grades, exposing the firm compacted inner core. The amount of overbuilding would vary depending on the site conditions at the time of construction, types of soils used, and degree of compaction achieved.

4. When placing fill in horizontal lifts adjacent to areas sloping steeper than 5:1 (horizontal: vertical), horizontal keys and vertical benches should be excavated into the adjacent slope area. Materials generated by the benching operation should be moved sufficiently away from the bench area to allow the geotechnical engineer (testing agency) to properly inspect the area and ascertain that the benching is performed properly.

5. We recommend that the fill be compacted to a minimum of 95% of the Standard Proctor Maximum Dry Density (ASTM Specifications D 698). The top 1 foot under pavements or structural areas should be compacted to a minimum of 98% of the Standard Proctor Test.

6. An experienced soil engineering inspector should take adequate density tests throughout the fill placement operation to ensure that the specified compaction is being achieved.

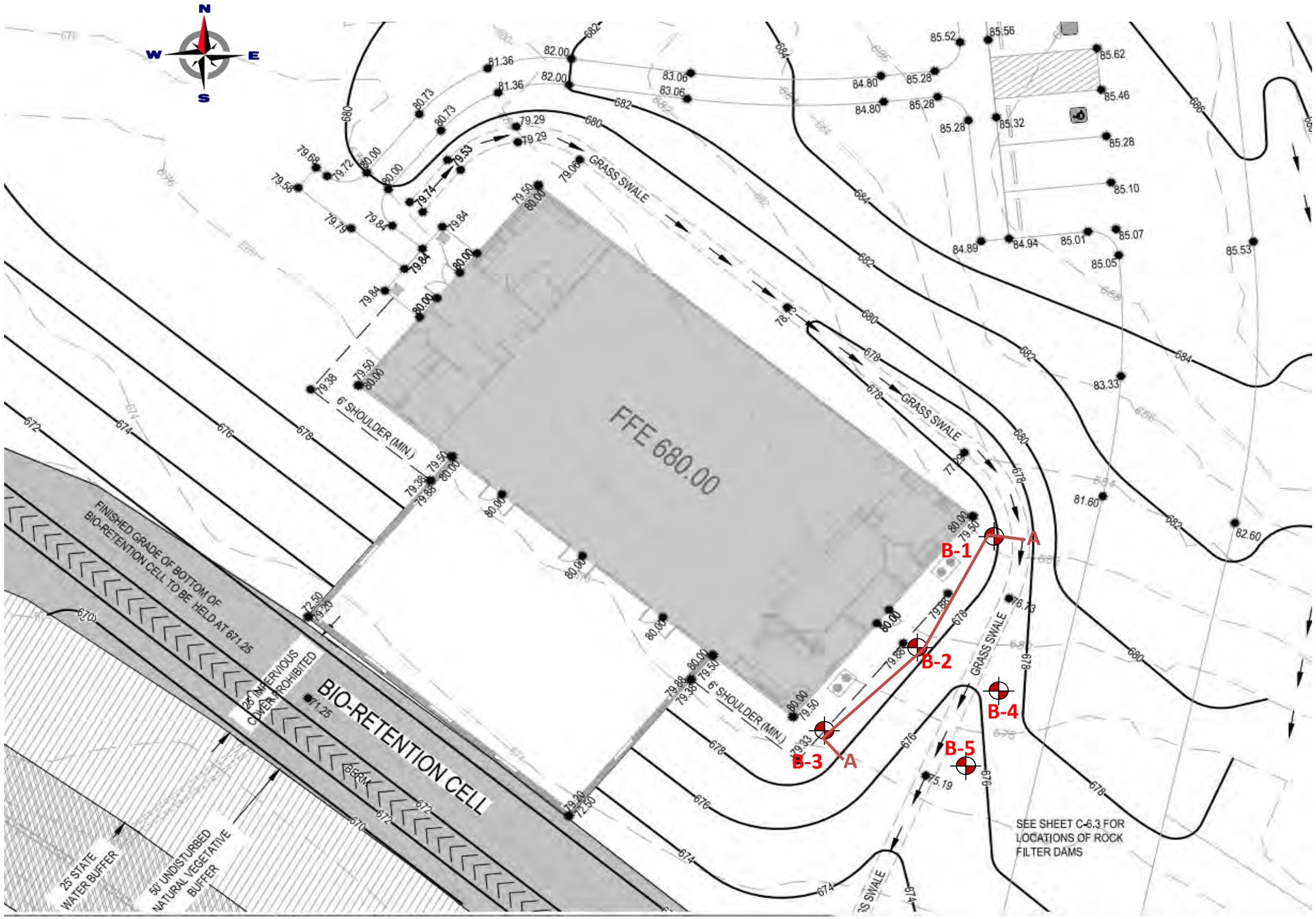
Matrix Engineering Group, Inc. appreciates the opportunity to have worked with you on this project and looks forward to our continued association. If you have any questions or need further assistance, please do not hesitate to call.

Sincerely,

MATRIX ENGINEERING GROUP, INC.

Sam Alyateem, PE
Principal Engineer

\\ZMATRIX\drive_m\1.0 MATRIX PROJECTS FOLDER\1.7 ROCKDALE COUNTY CONTRACT\Costly Mill Event Center Addition\1.1 Geotechnical - MEG 302287.61\Costley Mill Event Center Addition - Geotechnical Exploration_September 2024.doc



matrix engineering group

TITLE

Approximate Borings and Cross-Section Locations Plan

PROJECT

Cosltley Mill Event Center Addition

PROJECT #

MEG 302287.61

CLIENT

Rockdale County

SCALE

Not to Scale

REVIEWED

Sam Alyateem, PE

DATE

1/29/2024

FIGURE

1

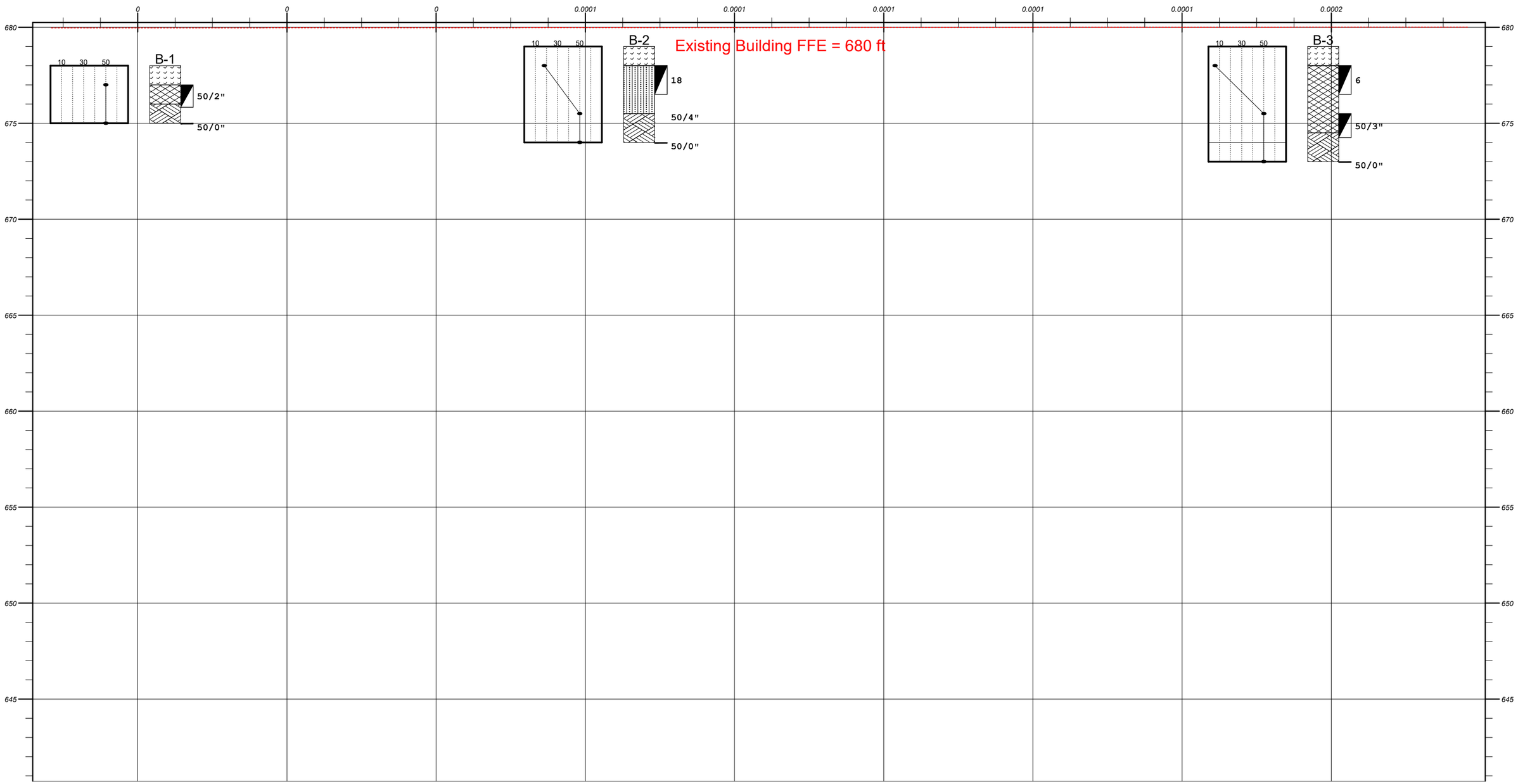
LEGEND

B-1 Boring Location

A / A Cross-Section Line

LOG OF BORINGS A-A

Costley Mill Event Center Addition



Top Soil

Fill

Partially Weathered Rock

Silty sand

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MAJOR DIVISIONS		SYMBOLS	TYPICAL NAMES	CLASSIFICATION CHART
COARSE-GRAINED SOILS (More Than 1/2 of Soil > #200 Sieve)	**GRAVELS** (More Than 1/2 of Coarse Fraction > #4 Sieve)	GW	Well Graded Gravels or Gravel-Sand Mixtures; Little or no fines	
		GP	Poorly Graded Gravels or Gravel-Sand Mixtures; Little or no fines	
		GM	Silty Gravels, Gravel-Sand-Silt Mixtures	
		GC	Clayey Gravels, Gravel-Sand-Clay Mixtures	
	SANDS (MORE Than 1/2 of Coarse Fraction < #4 Sieve)	SW	Well Graded Sands or Gravelly Sands; Little or no fines	
		SP	Poorly Graded Sands or Gravelly Sands; Little or no fines	
		SM	Silty Sands, Sand-Silt Mixtures	
		SC	Clayey Sands, Sand-Clay Mixtures	
FINE-GRAINED SOILS (More Than 1/2 of Soil < #200 Sieve)	**SILTS & CLAYS** Liquid Limit Less Than 50	ML	Inorganic Silts and Very Fine Sands, Rock Flour, Silty or Clayey Fine Sands or Clayey Silts with Slight Plasticity	
		CL	Inorganic Clays of Low to Medium Plasticity, Gravelly Clays, Sandy Clays, Silty Clays, Lean Clays	
		OL	Organic Silts and Organic Silty Clays of Low Plasticity	
	SILTS & CLAYS Liquid Limit Greater Than 50	MH	Inorganic Silts, Micaceous or Diatomaceous Fine Sandy or Silty Soils, Elastic Silts	
		CH	Inorganic Clays of High Plasticity, Fat Clays	
		OH	Organic Clays or Medium to High Plasticity, Organic Silty Clays, Organic Silts	
HIGHLY ORGANIC SOILS		PT	Peat and Other Highly Organic Soils	

Relative Density of Cohesionless Soils from Standard Penetration Test	
Very Loose	≤ 4 bpf
Loose	5-10 bpf
Medium Dense	11-30 bpf
Dense	31-50 bpf
Very Dense	> 50 bpf
(bpf=blows per foot; ASTM D1586)	

Consistency of Cohesive Soils	
Very Soft	≤ 2 bpf
Soft	3-4 bpf
Firm	5-8 bpf
Stiff	9-15 bpf
Very Stiff	16-30 bpf
Hard	30-50 bpf
Very Hard	> 50 bpf

Relative Hardness of Rock	
Very Soft	Hard rock disintegrates or easily compresses to touch; can be hard to very hard soil
Soft	May be broken with fingers
Moderately Soft	May be scratched with a nail, corners and edges may be broken with fingers
Moderately Hard	Light Blow of hammer required to break samples
Hard	Hard blow of hammer required to break sample

Particle Size Identification	
Boulders	Larger than 12"
Cobbles	3"-12"
Gravel	
Coarse	3/4"-3"
Fine	4.76mm-3/4"
Sand	
Coarse	2.0-4.76 mm
Medium	0.42-2.00 mm
Fine	0.42-0.074 mm
Fines (Silt or Clay)	Smaller than 0.074 mm

Rock Continuity	
RECOVERY (%) = Total Length of Core x 100 / Length of Core Run	
Description	Core Recovery (%)
Incompetent	Less than 40
Competent	40-70
Fairly Continuous	71-90
Continnuous	91-100

Relative Quality of Rocks	
RQD (%) =((Total core, counting only pieces >4" long)/(Length of Core Run)) x 100	
Description	RQD (%)
Very Poor	0-25
Poor	25-50
Fair	50-75
Good	75-90
Excellent	90-100

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Correlation of Penetration Resistance with Relative Density and Consistency Sheet and Soil Classification Chart

MATRIX ENGINEERING GROUP, INC.
Geotechnical, Environmental & Construction Materials Consultants

DRILL HOLE LOG

BORING NO. B-1

File: Borings Date Printed: 1/29/2024

PROJECT: Costley Mill Event Center Addition **PROJECT NO.:** MEG 302287.61
CLIENT: Rockdale County **DATE:** 1/26/2024
LOCATION: Refer to Figure 1 **ELEVATION:** 678 Feet MSL
DRILLER: Kilman Brothers **LOGGED BY:** Sulemana Alhassan
DRILLING METHOD: ASTM D1586 with Automatic Hammer **STATION:**
DEPTH TO - WATER> INITIAL: ▽ ____ **After 24+ Hours:** ▽ ____ **CAVING>** C ____

ELEVATION (feet)	DEPTH (feet)	Description	SOIL TYPE	SOIL SYMBOL	SAMPLERS	TEST RESULTS: Natural Moisture Content (%). ▲ Penetration - ● (10 20 30 40 50)	N-Value Blows/ft (ASTM D1586)
678	0						
		Approximately 4 inches Grass and Topsoil					
677	1						
		Fill - Loose, Brown, wet, Silty Sand.	Fill				50/2"
676	2						
		Partially Weathered Rock (PWR), Sampled as Very Dense, Mottled (Yellowish Orange and Light Gray), Silty SAND.	PWR				50/0"
675	3						
		Auger Refusal was encountered at 3.0 feet BGS.					
674	4						
673	5						
672	6						
671	7						
670	8						
669	9						
668	10						
667	11						
666	12						
665	13						
664	14						
663	15						
662	16						
661	17						
660	18						
659	19						
658	20						
657	21						
656	22						
655	23						
654	24						
653	25						
652	26						
651	27						
650	28						
649	29						

No Groundwater at the time of drilling. Borehole was backfilled at the conclusion of the field work.

MATRIX ENGINEERING GROUP, INC.
Geotechnical, Environmental & Construction Materials Consultants

DRILL HOLE LOG

BORING NO. B-2

File: Borings Date Printed: 1/29/2024

PROJECT: Costley Mill Event Center Addition **PROJECT NO.:** MEG 302287.61
CLIENT: Rockdale County **DATE:** 1/26/2024
LOCATION: Refer to Figure 1 **ELEVATION:** 679 Feet MSL
DRILLER: Kilman Brothers **LOGGED BY:** Sulemana Alhassan
DRILLING METHOD: ASTM D1586 with Automatic Hammer **STATION:**
DEPTH TO - WATER> INITIAL: ____ **After 24+ Hours:** ____ **CAVING>** C

ELEVATION (feet)	DEPTH (feet)	Description	SOIL TYPE	SOIL SYMBOL	SAMPLERS	TEST RESULTS (Natural Moisture Content (%). ▲ Penetration - ●)	N-Value Blows/ft (ASTM D1586)
679	0	Approximately 4 inches Grass and Topsoil					
678	1	Residual - Medium Dense, Mottled (Light Gray, Brown, and White), Silty SAND.	SM				18
677	2						
676	3						
675	4	Partially Weathered Rock (PWR), Sampled as Very Dense, Mottled (White, Greenish Gray, and Light Gray), Silty SAND.	PWR				50/4"
674	5						50/0"
673	6	Auger Refusal was encountered at 5.0 feet BGS.					
672	7						
671	8						
670	9						
669	10						
668	11						
667	12						
666	13						
665	14						
664	15						
663	16						
662	17						
661	18						
660	19						
659	20						
658	21						
657	22						
656	23						
655	24						
654	25						
653	26						
652	27						
651	28						
650	29						

No Groundwater at the time of drilling. Borehole was backfilled at the conclusion of the field work.

MATRIX ENGINEERING GROUP, INC.
Geotechnical, Environmental & Construction Materials Consultants

DRILL HOLE LOG

BORING NO. B-3

File: Borings Date Printed: 1/29/2024

PROJECT: Costley Mill Event Center Addition **PROJECT NO.:** MEG 302287.61
CLIENT: Rockdale County **DATE:** 1/26/2024
LOCATION: Refer to Figure 1 **ELEVATION:** 679 Feet MSL
DRILLER: Kilman Brothers **LOGGED BY:** Sulemana Alhassan
DRILLING METHOD: ASTM D1586 with Automatic Hammer **STATION:**
DEPTH TO - WATER> INITIAL: ▽ ____ **After 24+ Hours:** ▽ ____ **CAVING>** C ____

ELEVATION (feet)	DEPTH (feet)	Description	SOIL TYPE	SOIL SYMBOL	SAMPLERS	TEST RESULTS	N-Value Blows/ft (ASTM D1586)
679	0					Natural Moisture Content (%). ▲ Penetration - ● 10 20 30 40 50	
678	1	**Approximately 4 inches Grass and Topsoil**					
677	2	**Fill - Loose, Mottled (Brown and Dark Brown), Silty Sand with Hairline Roots and Gravel.**	Fill				6
676	3						
675	4						50/3"
674	5	**Partially Weathered Rock (PWR), Sampled as Very Dense, Silty SAND.**	PWR				
673	6						50/0"
		Auger Refusal was encountered at 6.0 feet BGS.					
672	7						
671	8						
670	9						
669	10						
668	11						
667	12						
666	13						
665	14						
664	15						
663	16						
662	17						
661	18						
660	19						
659	20						
658	21						
657	22						
656	23						
655	24						
654	25						
653	26						
652	27						
651	28						
650	29						

No Groundwater at the time of drilling. Borehole was backfilled at the conclusion of the field work.

MATRIX ENGINEERING GROUP, INC.
Geotechnical, Environmental & Construction Materials Consultants

DRILL HOLE LOG

BORING NO. B-4

File: Borings Date Printed: 1/29/2024

PROJECT: Costley Mill Event Center Addition **PROJECT NO.:** MEG 302287.61
CLIENT: Rockdale County **DATE:** 1/26/2024
LOCATION: Refer to Figure 1 **ELEVATION:** 676 Feet MSL
DRILLER: Kilman Brothers **LOGGED BY:** Sulemana Alhassan
DRILLING METHOD: ASTM D1586 with Automatic Hammer **STATION:**
DEPTH TO - WATER> INITIAL: ▽ ____ **After 24+ Hours:** ▼ ____ **CAVING>** C ____

ELEVATION (feet)	DEPTH (feet)	Description	SOIL TYPE	SOIL SYMBOL	SAMPLERS	TEST RESULTS (Natural Moisture Content (%). ▲ Penetration - ●)	N-Value Blows/ft (ASTM D1586)
676	0						
		Approximately 4 inches Grass and Topsoil					
675	1	**Auger Refusal was encountered at 1.0 feet BGS.**				● at 50	**50/0"**
674	2						
673	3						
672	4						
671	5						
670	6						
669	7						
668	8						
667	9						
666	10						
665	11						
664	12						
663	13						
662	14						
661	15						
660	16						
659	17						
658	18						
657	19						
656	20						
655	21						
654	22						
653	23						
652	24						
651	25						
650	26						
649	27						
648	28						
647	29						

10 20 30 40 50

No Groundwater at the time of drilling. Borehole was backfilled at the conclusion of the field work.

MATRIX ENGINEERING GROUP, INC.
Geotechnical, Environmental & Construction Materials Consultants

DRILL HOLE LOG

BORING NO. B-5

File: Borings Date Printed: 1/29/2024

PROJECT: Costley Mill Event Center Addition **PROJECT NO.:** MEG 302287.61
CLIENT: Rockdale County **DATE:** 1/26/2024
LOCATION: Refer to Figure 1 **ELEVATION:** 676 Feet MSL
DRILLER: Kilman Brothers **LOGGED BY:** Sulemana Alhassan
DRILLING METHOD: ASTM D1586 with Automatic Hammer **STATION:**
DEPTH TO - WATER> INITIAL: ▽ ____ **After 24+ Hours:** ▽ ____ **CAVING>** C ____

ELEVATION (feet)	DEPTH (feet)	Description	SOIL TYPE	SOIL SYMBOL	SAMPLERS	TEST RESULTS: Natural Moisture Content (%). ▲ Penetration - ● (10 20 30 40 50)	N-Value Blows/ft (ASTM D1586)
676	0						
		Approximately 4 inches Grass and Topsoil					
675	1						
		Residual - Medium Dense, Mottled (Light Gray and Yellowish Orange), Silty SAND.	SM				**17**
674	2						
673	3						**50/0"**
672	4	**Auger Refusal was encountered at 3.5 feet BGS.**					
671	5						
670	6						
669	7						
668	8						
667	9						
666	10						
665	11						
664	12						
663	13						
662	14						
661	15						
660	16						
659	17						
658	18						
657	19						
656	20						
655	21						
654	22						
653	23						
652	24						
651	25						
650	26						
649	27						
648	28						
647	29						

No Groundwater at the time of drilling. Borehole was backfilled at the conclusion of the field work.

matrix engineering group

Project Name: Costley Mill Event Center Addition	
Project Location: 2455 Costley Mill Rd NE, Conyers, GA 30013, USA	
Client: Rockdale County	Project Code: MEG 301287.61
Preparer: Sulemana Alhassan	Reviewer: Sam Alyateem, PE
Report Date: 01/29/2024	Page Number: 1 of 5

Picture 1: B-1

Weather: Fog

Lat/Lng: 33.7106,-83.9291
Bearing: S
Date Taken: 01/26/2024

Taken By: Sulemana J. A
Tags:

Picture 2: B-2

Weather: Light rain

Lat/Lng: 33.7105,-83.9293
Bearing: E
Date Taken: 01/26/2024

Taken By: Sulemana J. A
Tags:

Picture 3: B-3

Weather: Light rain

Lat/Lng: 33.7105,-83.9292
Bearing: NE
Date Taken: 01/26/2024

Taken By: Sulemana J. A
Tags:

Picture 4: B-4

Weather: Fog

Lat/Lng: 33.7105,-83.9291
Bearing: W
Date Taken: 01/26/2024

Taken By: Sulemana J. A
Tags:

matrix engineering group

Project Name: Costley Mill Event Center Addition

Project Location: 2455 Costley Mill Rd NE, Conyers, GA 30013, USA

Client: Rockdale County

Project Code: MEG 301287.61

Preparer: Sulemana Alhassan

Reviewer: Sam Alyateem, PE

Report Date: 01/29/2024

Picture 5: B-5

Weather: Fog

Lat/Lng: 33.7106,-83.9291
Bearing: W
Date Taken: 01/26/2024

Taken By: Sulemana J. A
Tags:

Picture 6:

Weather: Overcast

Lat/Lng: 33.7105,-83.9292
Bearing: N
Date Taken: 01/23/2024

Taken By: Sulemana J. A
Tags:

Picture 7:

Weather: Overcast

Lat/Lng: 33.7104,-83.9292
Bearing: NW
Date Taken: 01/23/2024

Taken By: Sulemana J. A
Tags:

Picture 8:

Weather: Overcast

Lat/Lng: 33.7106,-83.9290
Bearing: NW
Date Taken: 01/23/2024

Taken By: Sulemana J. A
Tags:

matrix engineering group

Project Name: Costley Mill Event Center Addition

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Preparer: Sulemana Alhassan

Reviewer: Sam Alyateem, PE

Report Date: 01/29/2024

Picture 9:

Weather: Overcast

Lat/Lng: 33.7107,-83.9289
Bearing: W
Date Taken: 01/23/2024

Taken By: Sulemana J. A
Tags:

Picture 10:

Weather: Overcast

Lat/Lng: 33.7108,-83.9290
Bearing: SW
Date Taken: 01/23/2024

Taken By: Sulemana J. A
Tags:

Picture 11:

Weather: Overcast

Lat/Lng: 33.7110,-83.9291
Bearing: W
Date Taken: 01/23/2024

Taken By: Sulemana J. A
Tags:

Picture 12:

Weather: Overcast

Lat/Lng: 33.7108,-83.9290
Bearing: W
Date Taken: 01/23/2024

Taken By: Sulemana J. A
Tags:

matrix engineering group

Project Name: Costley Mill Event Center Addition

Project Location: 2455 Costley Mill Rd NE, Conyers, GA 30013, USA

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Reviewer: Sam Alyateem, PE

Report Date: 01/29/2024

Picture 13:

Weather: Light rain

Lat/Lng: 33.7105,-83.9292
Bearing: E
Date Taken: 01/26/2024

Taken By: Sulemana J. A
Tags:

Picture 14:

Weather: Light rain

Lat/Lng: 33.7105,-83.9291
Bearing: NW
Date Taken: 01/26/2024

Taken By: Sulemana J. A
Tags:

Picture 15:

Weather: Fog

Lat/Lng: 33.7105,-83.9289
Bearing: SW
Date Taken: 01/26/2024

Taken By: Sulemana J. A
Tags:

matrix engineering group

Project Name: Costley Mill Event Center Addition

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