

**LIMITED GEOTECHNICAL EXPLORATION  
SOIL TEST BORINGS**

**For**

***Playground, Pavilion, Volleyball Courts, and Splash Pad  
Wheeler Park  
1400 Parker Road SE, Conyers, GA 30094***

*Submitted to*

**Ms. Ronda Harston  
General Services Projects Coordinator  
Dept. of General Services  
Rockdale County Government  
1329 Portman Dr., Suite J  
Conyers, GA 30094**

**April 2023**

**MEG 301287.46**





April 10, 2023

Ms. Ronda Harston  
General Services Projects Coordinator  
Dept. of General Services  
Rockdale County Government  
1329 Portman Dr., Suite J  
Conyers, GA 30094

**Re: *Limited Geotechnical Exploration – Soil Test Borings  
Playground, Pavilion, Volleyball Courts, and Splash Pad @ Wheeler Park  
Matrix Engineering Group Project Number MEG-301287.46***

Dear Ms. Harston:

Matrix Engineering Group, Inc. has completed the authorized Limited Geotechnical Exploration for the Rockdale County Wheeler Park project located at 1400 Parker Road in Conyers, Georgia 30094.

The scope of this work was to perform a total of twelve (12) soil test borings within the proposed construction area and determine the presence of rock within the top ten (10) feet of the existing ground surface.

This work was performed in general accordance with Matrix Proposal Number 030223-1, dated March 02, 2023, and the subsequent authorization to proceed by you on March 23, 2023. This report describes our exploration procedures and presents our findings and recommendations.

## **INTRODUCTION**

The site is within the existing Rockdale County Wheeler Park located at 1400 Parker Road in Conyers, Georgia 30094. The proposed development area is located south of the existing Restoration Storehouse Center building. We noted an Outdoor Fitness Center northeast of the site. The ground surface of the site is covered with vegetation/grass.

Based on the Concept Plan dated 01-23-2023 and our site visit, the subject site slopes gently in a southerly direction from an approximate elevation of 894 feet Mean Sea Level (MSL) at the Restoration Storehouse Center building to an approximate elevation of 890 feet MSL. The topography then slopes sharply from

890 feet MSL to an approximate elevation of 886 feet MSL and then slopes down gently to an approximate elevation of 882 feet MSL at the southern end of the site.

## **EXPLORATION AND TESTING PROGRAM**

The geotechnical exploration program consisted of drilling a total of twelve (12) soil test borings at the proposed site. The approximate locations of the soil borings are shown on Figure 1 presented in the Appendix of this report. For exact locations, the owner may elect to survey the boring locations. Matrix should be informed of any deviations in order to evaluate and modify our recommendations, if necessary.

The test borings were performed utilizing a track rig mounted with a GeoProbe drilling apparatus equipped with an automatic hammer in general accordance with ASTM D1586 standards. The planned depth of the borings was 10 feet BGS. Borings were advanced by auguring through the soils with continuous flights of 3 1/4-inch ID augers. At the depth of 3.5 feet to 5 feet, soil samples were obtained through the center of the auger flights with a standard 1.4-inch I.D., 2-inch O.D., split-tube sampler. The sampler is first seated 6 inches to penetrate loosened strata before sampling, and then driven an additional foot with blows of a 140-pound hammer falling 30 inches. The number of hammer blows required to drive the sampler the final foot is recorded and is designated as the Standard Penetration Resistance (N-Value). The penetration resistance, when properly evaluated, is an index of the soil strength, consistency and ability to support foundations. The boring was then augured (without sampling) to the termination depth of 10 feet BGS.

Representative soil samples were obtained using split-spoon sampling techniques. The samples were classified in the field in general accordance with ASTM D2488 (Visual-Manual Procedure for Description of Soils). Representative portions of the soil samples were placed in sealable, plastic jars and transported to our laboratory. During the field operations, Matrix staff maintained a continuous log of the subsurface conditions including changes in the stratigraphy and any observed groundwater levels. Soil descriptions and penetration resistance values are presented graphically on the Soil Boring Records included in the Appendix of this report.

All borings were backfilled with the soil cuttings by Matrix Engineering at the conclusion of the fieldwork. Some consolidation of the backfilled soil column should be expected over time.

## **GENERAL SITE GEOLOGY**

The subject site is in the Piedmont Geologic Province, which contains the oldest rock formations in the Southeastern United States. The parent rocks in the region are primarily comprised of the unconsolidated mass of quartz, feldspar, mica, and a wide variety of dark minerals such as hornblende and amphibole. The proportion of felsic and mafic minerals in these parent rocks, as well as of quartz that is very resistant to weathering, limits the amount of clay in the soils. Therefore, these soils are sandy and have faint horizons, and in small-scattered areas, hard rock is exposed.

Chemical decomposition initially occurs along the boundaries of individual mineral crystals. As a result, partially weathered rock has the appearance of dense sand (SM, SP). With further weathering, the individual crystals other than quartz are attacked and the mass becomes a micaceous silty sand (SM) or micaceous sandy silt (ML). In this stage, the original banding of the parent rock is apparent, but the original crystalline structure is not observed. Reflecting the composition of the original rock, mica flakes, rather than the quartz grains, often comprise the majority of the sand-size particles. Finally, in the more advanced stages of chemical weathering, the material is changed into a red or reddish-brown silty clay (CL or CH) or clayey silt (ML or MH). Depending on the quartz content, a sandy fraction will be present. In this weathered stage, the banding and crystalline structure of the parent rocks is lost.

## **GENERAL SUBSURFACE CONDITIONS**

The subsurface conditions were characterized by visual-manual examination of the soils obtained from the soil test borings and observations from the auger cuttings during the auguring operations. The soil boring logs, designated as B-1 to B-12, are provided in the Appendix of this report. The subsurface conditions within the soil test borings are characterized as follows:

The borings encountered approximately 4 inches of grass and topsoil. Topsoil thickness may vary elsewhere, and the reported thicknesses should not be used to estimate the amount of stripping that will be necessary to properly prepare the site for structural fill. Additionally, the term topsoil should not connote a horticultural (or agricultural) definition or classification, but rather a visually determined organic-laden material.

Man-made fill soils were encountered within the top 3.5 feet at some of the test borings. The fill soil was very firm inorganic Clayey and Silty Sand. Residual soils were encountered beneath the fill layer or the surficial cover and consisted of very firm silty sand up to the termination depth of ten (10) feet below the existing ground surface (BGS). The penetration resistance, N-values, within the man-made fill and residual soils ranging from 7 to 15 bpf.

No Partially Weathered Rock (PWR); nor rock boulder was encountered within the drilled depth. Groundwater was also not encountered within the drilled depth at the time of drilling. Refer to the Appendix of this report for the boring logs and the soil profiles.

## RECOMMENDATIONS

*The following recommendations are based on the information furnished to us, the data obtained from the subsurface exploration, and our experience with similar projects. They were prepared in general accordance with established and accepted professional geotechnical engineering practice in this region. Our recommendations are based on findings from the dates referenced within this report and do not reflect any variations that would likely exist at later dates or between the pre-designated borings or unexplored areas.*

*If information becomes available which may impact our recommendations, Matrix Engineering Group shall be afforded the opportunity to review this information and re-evaluate the recommendations contained within this report and make any alterations deemed necessary by a Georgia Registered professional engineer. This report is intended for the use of Rockdale County and its current design team. No other warranty is expressed or implied. Matrix Engineering Group, Inc. is not responsible for conclusions, opinions, or recommendations made by others based on this report.*

### General Site Preparation

Site preparation for the proposed development will include stripping of topsoil and soft soils, where encountered. Any debris or other items, such as underground utility lines, or trash pits that may be encountered during the grading operation should be brought treated on an individual basis and brought up to the attention of the Geotechnical Engineer for evaluation and recommendations.

Based on the Concept Site Plan dated 01/23/2023, the topographic relief within the footprint of the proposed Splash Pad is approximately 2 feet with elevations between 882 feet MSL and 884 feet MSL. The topographic relief for the Volleyball Courts is approximately 3 feet with elevations between 882 feet MSL and 885 feet MSL. The proposed Picnic Pavilion and the Playground are at a relatively flat area with elevations at approximately 891 feet MSL.

Since concrete stairs will separate the Splash Pad/Volleyball areas from the Picnic Pavilion/Playground areas, it appears that minimum cut and fill will be required to prepare the site to the desired finished elevations. We recommend that any material which is excavated and planned for re-use as structural fill be examined by the geotechnical engineer of record at the time of excavation in order to determine its suitability. Fill soils should be free of organics, construction debris, cobbles, or other deleterious materials.

In general, soil encountered within the drilled depths appears to be suitable for use as a structural fill. Adequate laboratory testing should be performed during construction in order to ensure that the fill materials within all structural areas be suitable to support the proposed structures. Refer to the Structural fill procedures section provided in this report.

### **Subgrade Preparation**

Subgrade preparation for the proposed development should be the stripping of vegetation and topsoil and any deleterious materials, if encountered. Topsoil can be used in proposed landscape areas.

After removal of the surface materials, the suitability of the exposed subgrade should be confirmed by proofrolling at the time of construction, which will discern any localized soft zones in the subgrade. The proofrolling should be performed by a loaded tandem-wheeled dump truck with an approximate weight of 25 tons. Any material that deflects excessively or ruts under the loaded truck should be densified or removed and replaced with well-compacted material. The proofrolling should be observed by the geotechnical engineer.

After the subgrades are approved, structural fill may proceed in accordance with the project specification or meet the minimum requirements provided in this report.

### **Foundations**

The site appears to be suitable for the proposed development. The proposed Picnic Pavilion may be supported on shallow foundations. The foundations should be situated in undisturbed soil or structural fill placed in accordance with the recommended criterion provided in this report. All bearing soil should be evaluated by the geotechnical engineer and inspected in accordance with the criterion provided in this report.

We recommend that the foundations be designed for a maximum net allowable soil bearing pressure not to exceed **2,000 pounds per square foot (psf)**.

If soft or unsuitable soils are encountered during the foundation excavation, undercutting of unsuitable and/or soft soils and backfill with suitable soils or crushed stone may be performed to achieve the recommended bearing capacity.

### **Concrete Sidewalk**

The concrete sidewalk should be supported on compacted, and properly prepared soil subgrades. Provided that the fill material and/or existing subgrade is installed to a minimum of 98% of the Standard Proctor's maximum dry density, a modulus of subgrade reaction (k) of 100 pci can be used for designing the concrete pavements. Control joints and Construction joints should be carefully placed to minimize random shrinkage cracks. The spacing of the joints typically depends on the mix design, width of the trail, reinforcement, and thickness of the concrete slab. We recommend that maximum spacing for control joints be 10 feet and expansion joints be a maximum of 75 feet.

### **Slab-on-Grade**

The concrete slab-on-grade for the proposed structure(s) should be supported on compacted, and properly prepared, soil subgrade. Provided the fill material and/or existing subgrade is installed to a minimum of 95% of the Standard Proctor's maximum dry density, a modulus of subgrade reaction (k) of 100 pci can be used for designing the floor slab-on-grade. Slab reinforcement and joint spacing should be carefully considered to control random cracking due to slab shrinkage. We recommend that a 10 mil vapor barrier/retarder (such as polyethylene) be installed below the (slab-on-grade) concrete to limit intrusion of water vapor through the slab. Beneath slab-on-grade areas, a minimum of 4 inches of clean, densely graded, granular material with a balanced content of fines is recommended to facilitate fine grading and provide stable surface for construction traffic and building loads. Open-graded bases (such as #57 stone) do not meet these requirements because they are relatively incompatible, difficult to trim, and are unstable for construction traffic. It is also difficult to fine grade an open-graded base to a relatively uniform elevation, which can result in restraint to concrete movement as the concrete cools or dries, thus increasing the probability of out-of-joint cracking. If open-graded bases are specified, the surface of these bases should be choked off with a clean fine-graded material with at least 10 to 30% of the particles passing a No. 100 sieve, but not contaminated with clay, silt, or organic material.

### **Structural Fill**

Staged, methodical and well-planned grading is key to avoiding unnecessary costs and time delays. Areas should not be stripped or disturbed if the grading contractor is unable to properly seal the subgrade prior to departure each day. Exposure of soils to moisture from direct rainfall or runoff usually renders these soils unusable for several days. This usually gets mischaracterized as an unsuitable soils condition which is inaccurate. Unsuitable soils are defined as those containing deleterious matter (such as organics, alluvium, debris and/or trash). Moisture related problems should be avoided by employing best management practices that involve

maintaining positive drainage, installation of berms, diversion channels, and/or sealing the subgrade to avoid water infiltration. Other measures involve covering all stockpiled soils with heavy tarps or plastic to avoid saturating the soils in the event of rainfall. Means and methods of construction are certainly the contractor's jurisdiction; however, exposing otherwise suitable soils to excessive moisture or softening of existing subgrades as a result of unscrupulous construction traffic should be avoided and planned for.

We recommend that the following criteria be used for structural fill:

1. Adequate laboratory proctor density tests should be performed on representative samples of the proposed fill materials to provide data necessary for the quality control. The moisture content at the time of compaction should be within 3 percentage points of the optimum moisture content. In addition, we recommend that the fill soils be free of organics and rock boulder/cobbles larger than 2 inches in nominal size and relatively non-plastic with plasticity indices less than 20.
2. Suitable fill material should be placed in thin lifts (lift thickness depends on type of equipment used, but generally lifts of 8 inches loose measurements are recommended). The soils should be compacted by mechanical means such as sheepfoot rollers.
3. Slopes that are limited to 2:1 (horizontal: vertical), or flatter, will have adequate long term slope stability, if limited in height to 15 feet, based on our experience with the type of soils encountered onsite. The slope's crest should be protected against water ponding. Proposed slopes should incorporate only suitable fill, clean of organics or any other vegetative content. Topsoil should only be used to provide cover over the completed slope's free face so as to promote vegetative growth which in turn protects the slope's surface against scour and erosion. Slopes should be overbuilt and cut back to the proposed grades, exposing the firm compacted inner core. The amount of overbuilding would vary depending on the site conditions at the time of construction, types of soils used and degree of compaction achieved.
4. When placing fill in horizontal lifts adjacent to areas sloping steeper than 5:1 (horizontal:vertical), horizontal keys and vertical benches should be excavated into the adjacent slope area. Materials generated by the benching operation should be moved sufficiently away from the bench area to allow the geotechnical engineer (testing agency) to properly inspect the area and ascertain that the benching is performed properly.



5. We recommend that the fill be compacted to a minimum of 95% of the Standard Proctor Maximum Dry Density (ASTM Specifications D 698). The top 2 feet under pavements or structural areas should be compacted to a minimum of 98% of the Standard Proctor Test.
  
6. An experienced soil engineering inspector should take adequate density tests throughout the fill placement operation to ensure that the specified compaction is being achieved.

### **Inspection and Testing**

During construction, we recommend that Matrix Engineering Group inspect the site preparation and foundation construction work in order to ensure that our recommended procedures are followed. The placement of any compacted fill should be inspected and tested. The utilization of acceptable on-site borrow materials, as well as adequate off-site selected fill must be verified.

Each footing excavation should be inspected by Matrix Engineering Group, Inc. in order to verify the availability of the required bearing pressure and to determine any special procedures required. At a minimum, Hand Auger and Dynamic Cone Penetrometer testing in accordance with ASTM STP 399 should be performed at each shallow column footing, and every 50 linear feet for wall footings, or as directed by the geotechnical engineer in order to:

- Verify materials below footings are adequate to achieve the designed bearing capacity.
- Verify excavations are extended to proper depths and have reached proper material.
- Perform classification and testing of controlled fill materials.
- Verify use of proper materials, densities and lift thicknesses during placement and compaction of controlled fill.
- Prior to placement of controlled fill, observe subgrade and verify that the site has been properly prepared.

Matrix Engineering Group, Inc. appreciates the opportunity to have worked with you on this project and looks forward to our continued association. If you have any questions or need further assistance, please do not hesitate to call.

Best Regards,

**MATRIX ENGINEERING GROUP, INC.**



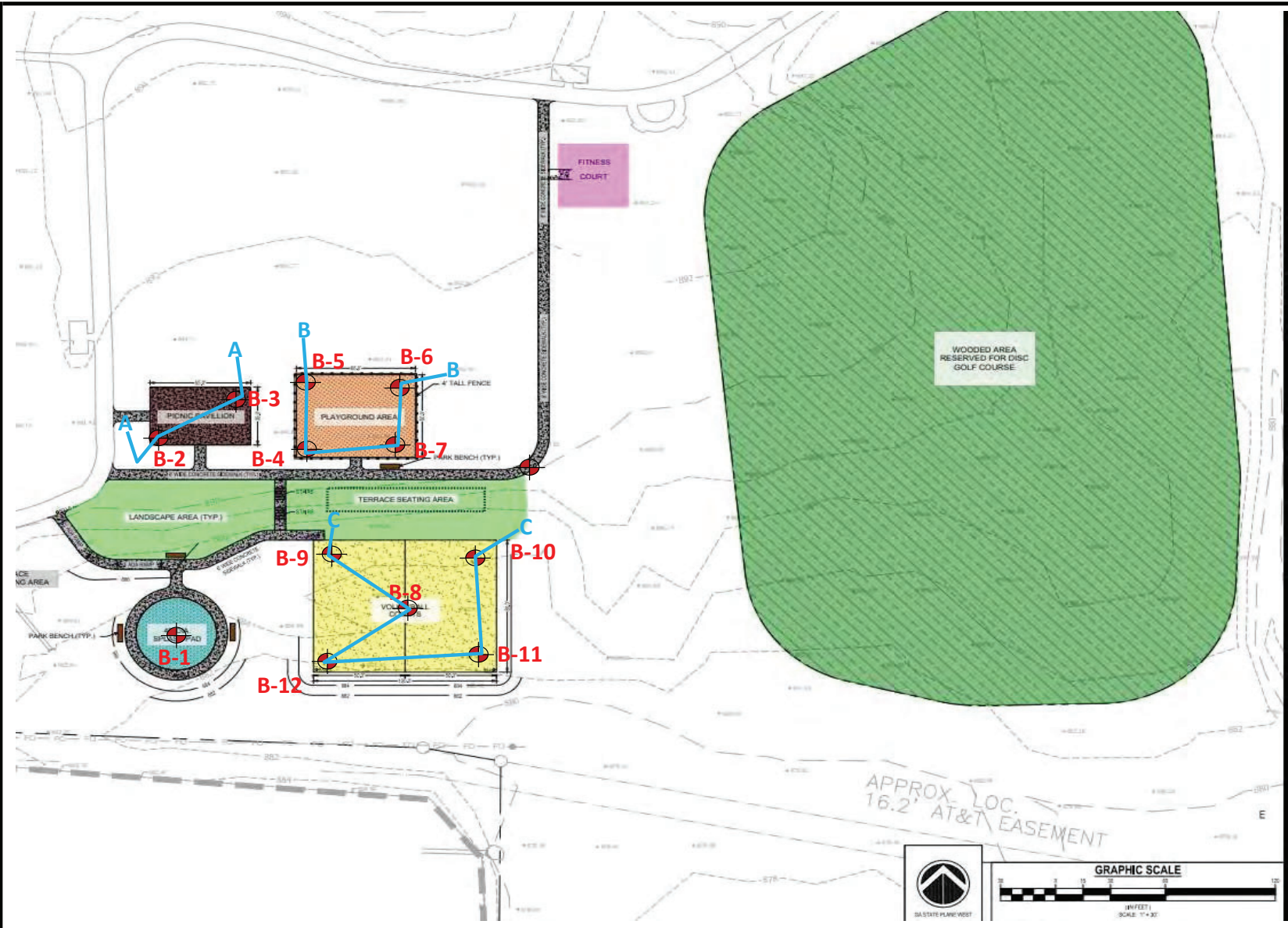
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Sam Alyateem, PE  
Senior Geotechnical Engineer  
Principal

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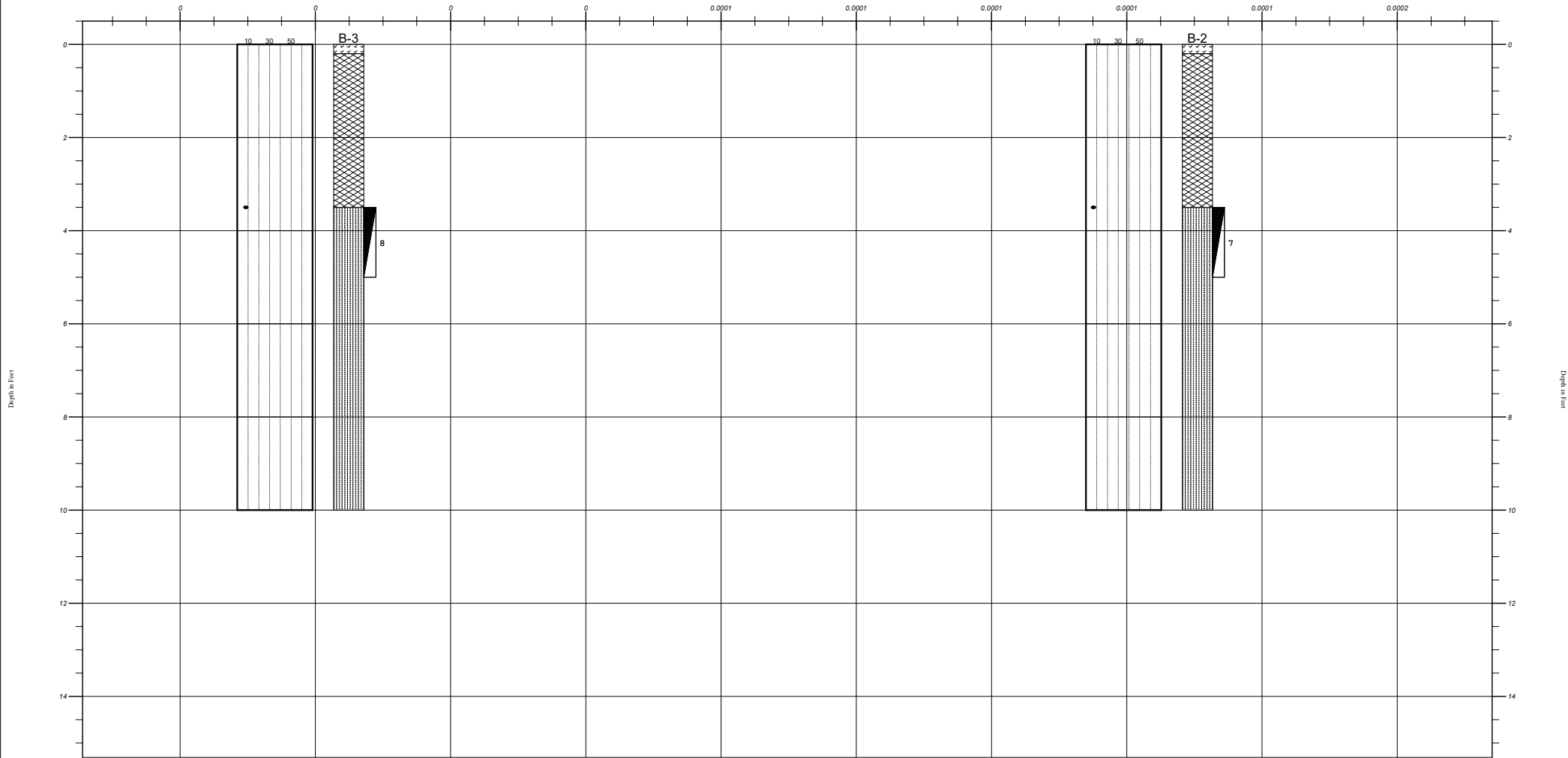





**matrix**  
engineering group

TITLE	Approximate Soil test Locations Plan
PROJECT	Rockdale County Wheeler Park
PROJECT #	MEG 301287.46
CLIENT	Rockdale County Government
SCALE	Not to Scale
PREPARED BY	Sulemana Alhassan
REVIEWED BY	Sam Alyateem, PE
DATE	3/7/2023
FIGURE	1
LEGEND	<ul style="list-style-type: none"> <li> <b>B-1</b> Boring Locations</li> <li> Cross-Section Line</li> </ul>

# LOG OF BORINGS A-A

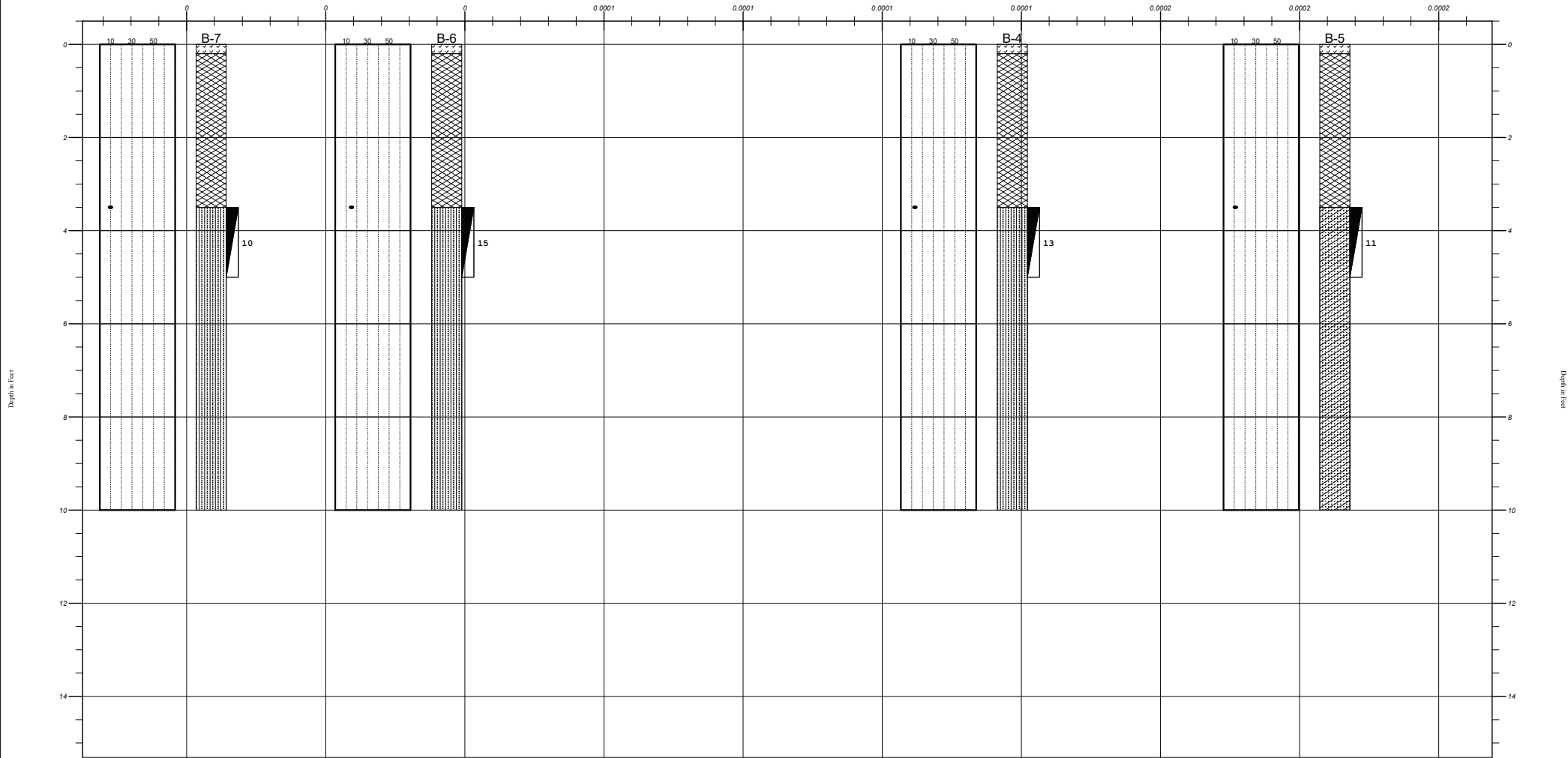
Wheeler Park - Playground, Pavilion, Volley Ball Courts, and Splash Pad.



-  Top Soil
-  Fill
-  Silty sand

# LOG OF BORINGS B-B

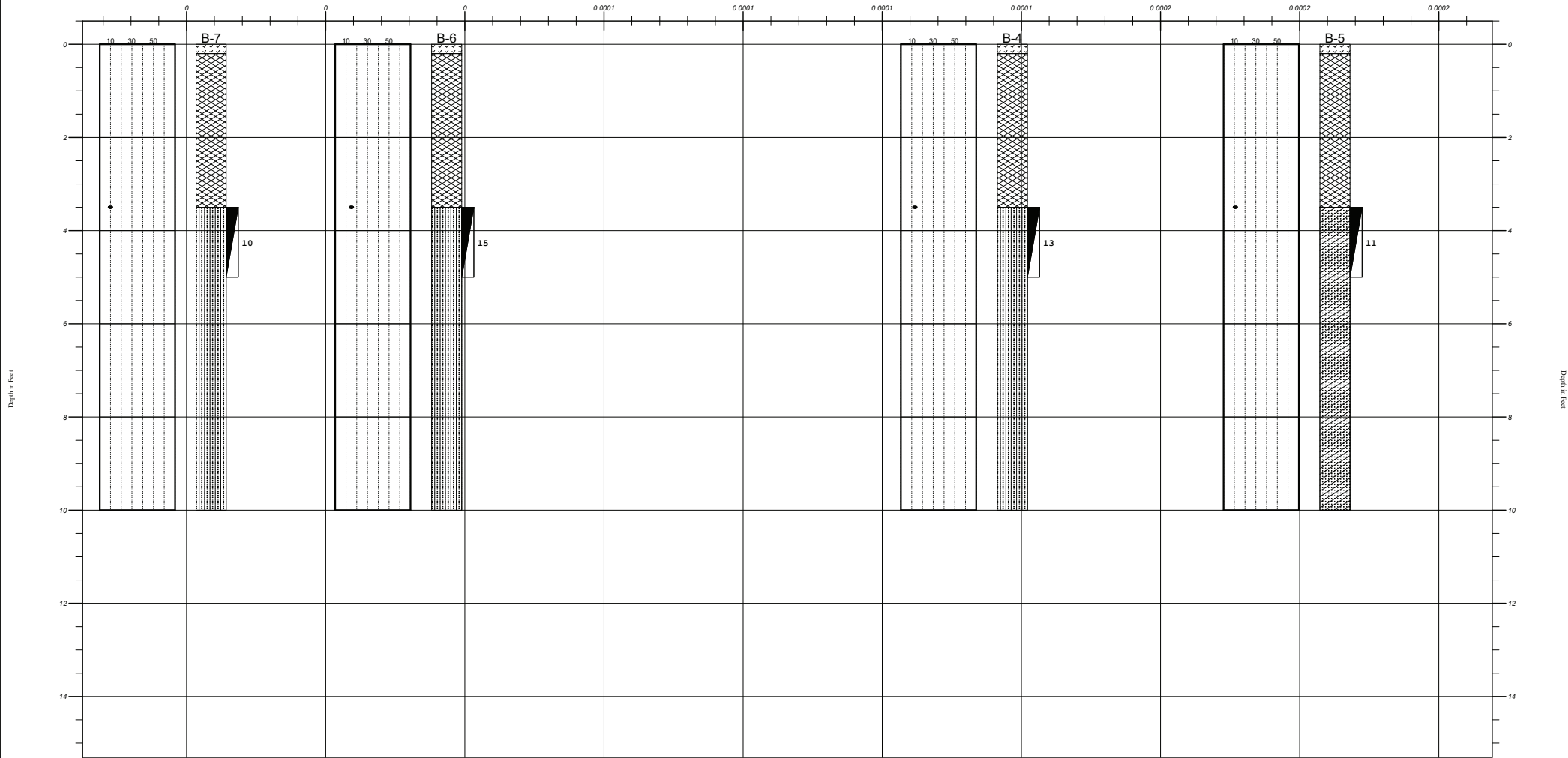
Wheeler Park - Playground, Pavilion, Volley Ball Courts, and Splash Pad.



- Top Soil
- Clayey sand/  
Low plasticity clay
- Fill
- Silty sand

# LOG OF BORINGS B-B

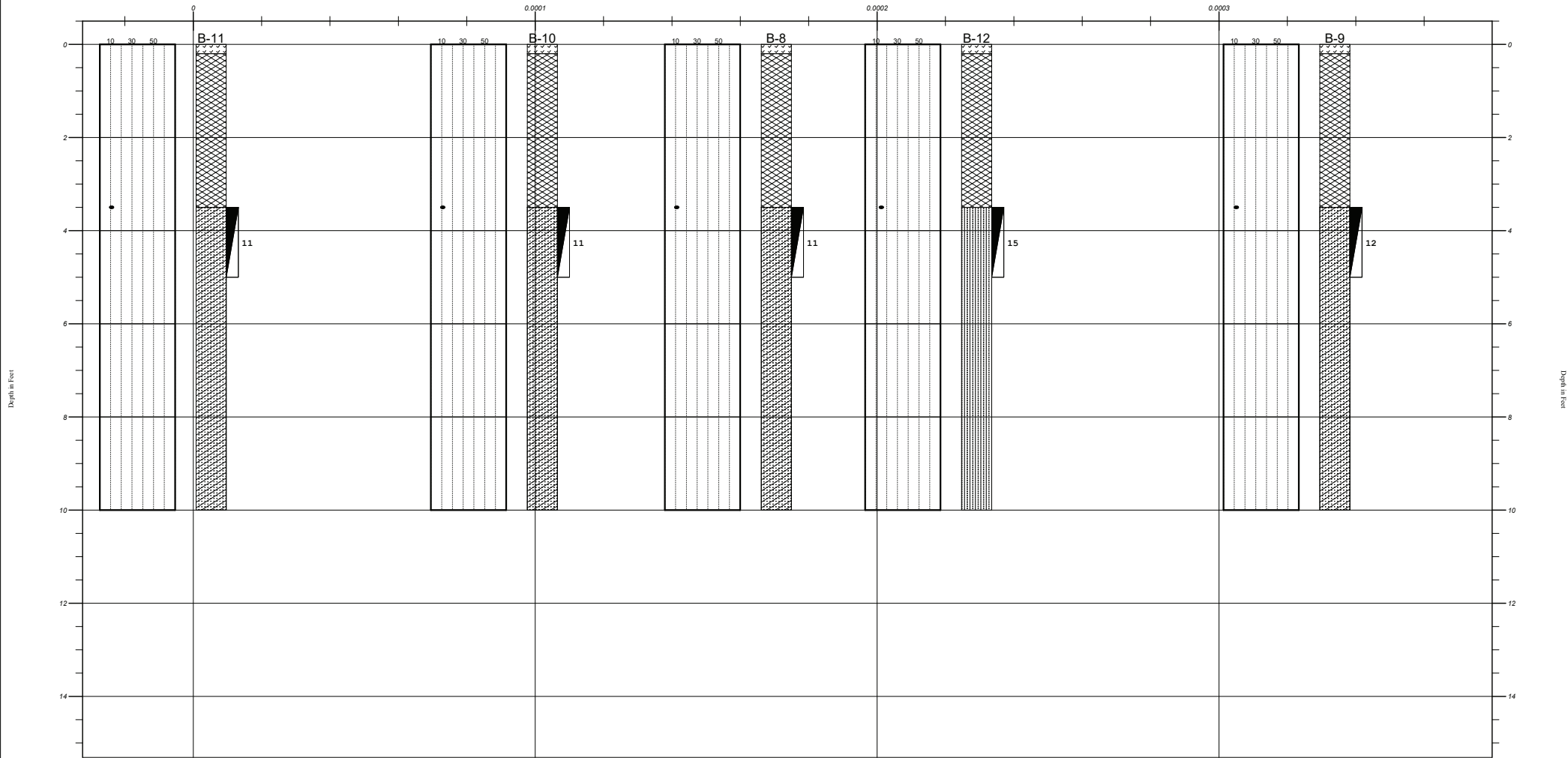
Wheeler Park - Playground, Pavilion, Volley Ball Courts, and Splash Pad.



-  Top Soil
-  Fill
-  Silty sand
-  Clayey sand/  
Low plasticity clay

# LOG OF BORINGS C-C

Wheeler Park - Playground, Pavilion, Volley Ball Courts, and Splash Pad.



- Top Soil
- Fill
- Clayey sand/  
Low plasticity clay
- Silty sand

MAJOR DIVISIONS		SYMBOLS	TYPICAL NAMES
COARSE-GRAINED SOILS (More Than 1/2 of Soil > #200 Sieve)	<b>GRAVELS</b> (More Than 1/2 of Coarse Fraction > #4 Sieve)	GW	Well Graded Gravels or Gravel-Sand Mixtures; Little or no fines
		GP	Poorly Graded Gravels or Gravel-Sand Mixtures; Little or no fines
		GM	Silty Gravels, Gravel-Sand-Silt Mixtures
		GC	Clayey Gravels, Gravel-Sand-Clay Mixtures
	<b>SANDS</b> (MORE Than 1/2 of Coarse Fraction < #4 Sieve)	SW	Well Graded Sands or Gravelly Sands; Little or no fines
		SP	Poorly Graded Sands or Gravelly Sands; Little or no fines
		SM	Silty Sands, Sand-Silt Mixtures
		SC	Clayey Sands, Sand-Clay Mixtures
FINE-GRAINED SOILS (More Than 1/2 of Soil < #200 Sieve)	<b>SILTS &amp; CLAYS</b> Liquid Limit Less Than 50	ML	Inorganic Silts and Very Fine Sands, Rock Flour, Silty or Clayey Fine Sands or Clayey Silts with Slight Plasticity
		CL	Inorganic Clays of Low to Medium Plasticity, Gravelly Clays, Sandy Clays, Silty Clays, Lean Clays
		OL	Organic Silts and Organic Silty Clays of Low Plasticity
	<b>SILTS &amp; CLAYS</b> Liquid Limit Greater Than 50	MH	Inorganic Silts, Micaceous or Diatomaceous Fine Sandy or Silty Soils, Elastic Silts
		CH	Inorganic Clays of High Plasticity, Fat Clays
		OH	Organic Clays or Medium to High Plasticity, Organic Silty Clays, Organic Silts
<b>HIGHLY ORGANIC SOILS</b>	PT	Peat and Other Highly Organic Soils	

CLASSIFICATION CHART

Relative Density of Cohesionless Soils from Standard Penetration Test	
Very Loose	≤ 4 bpf
Loose	5-10 bpf
Medium Dense	11-30 bpf
Dense	31-50 bpf
Very Dense	> 50 bpf
(bpf=blows per foot; ASTM D1586)	

Consistency of Cohesive Soils	
Very Soft	≤ 2 bpf
Soft	3-4 bpf
Firm	5-8 bpf
Stiff	9-15 bpf
Very Stiff	16-30 bpf
Hard	30-50 bpf
Very Hard	> 50 bpf

Relative Hardness of Rock	
Very Soft	Hard rock disintegrates or easily compresses to touch; can be hard to very hard soil
Soft	May be broken with fingers
Moderately Soft	May be scratched with a nail, corners and edges may be broken with fingers
Moderately Hard	Light Blow of hammer required to break samples
Hard	Hard blow of hammer required to break sample

Particle Size Identification	
Boulders	Larger than 12"
Cobbles	3"-12"
Gravel	
Coarse	3/4"-3"
Fine	4.76mm-3/4"
Sand	
Coarse	2.0-4.76 mm
Medium	0.42-2.00 mm
Fine	0.42-0.074 mm
Fines (Silt or Clay)	Smaller than 0.074 mm

Rock Continuity	
RECOVERY (%) = $\frac{\text{Total Length of Core} \times 100}{\text{Length of Core Run}}$	
Description	Core Recovery (%)
Incompetent	Less than 40
Competent	40-70
Fairly Continuous	71-90
Continuous	91-100

Relative Quality of Rocks	
RQD (%) = $\frac{(\text{Total core, counting only pieces } > 4" \text{ long})}{(\text{Length of Core Run})} \times 100$	
Description	RQD (%)
Very Poor	0-25
Poor	25-50
Fair	50-75
Good	75-90
Excellent	90-100



**Matrix  
Engineering  
Group, Inc.**

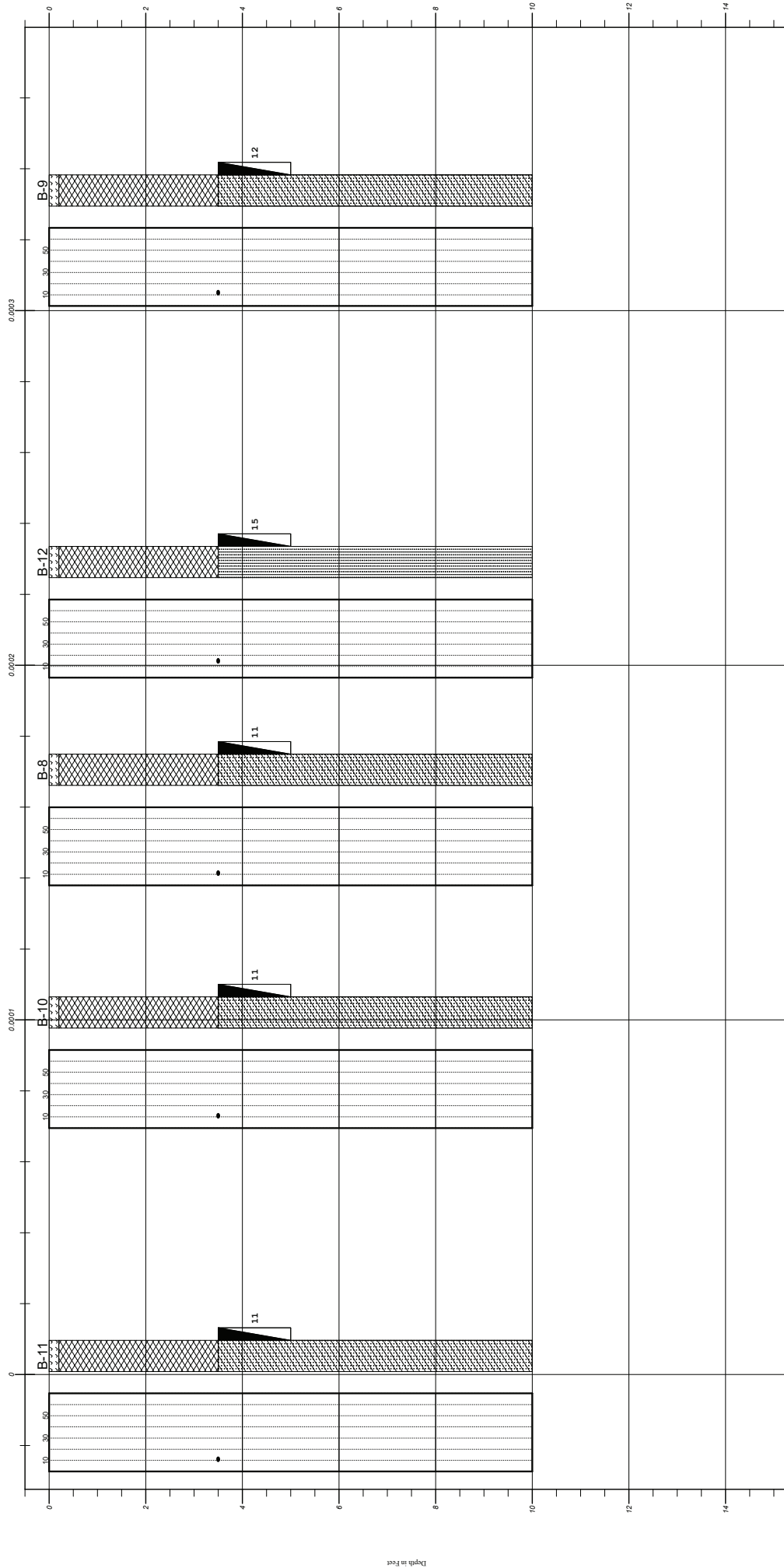
engineers | special inspectors | construction consultants

**Correlation of Penetration Resistance with Relative Density and Consistency Sheet and Soil Classification Chart**



# LOG OF BORINGS C-C

## Wheeler Park - Playground, Pavilion, Volley Ball Courts, and Splash Pad.



MAJOR DIVISIONS		SYMBOLS	TYPICAL NAMES
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FINE-GRAINED SOILS (More Than 1/2 of Soil < #200 Sieve)	SANDS & CLAYS (MORE Than 1/2 of Coarse Fraction < #4 Sieve)	SW	Well Graded Sands or Gravelly Sands; Little or no fines
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HIGHLY ORGANIC SOILS	SILTS & CLAYS Liquid Limit Greater Than 50	CH	Inorganic Clays of High Plasticity, Fat Clays
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		PT	Peat and Other Highly Organic Soils

**CLASSIFICATION CHART**

Relative Density of Cohesionless Soils from Standard Penetration Test	
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Very Dense	> 50 bpf
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Particle Size Identification	
Boulders	Larger than 12"
Cobbles	3"-12"
Gravel	Coarse 3/4"-3" Fine 4.76mm-3/4"
Sand	Coarse 2.0-4.76 mm Medium 0.42-2.00 mm Fine 0.42-0.074 mm
Fines (Silt or Clay)	Smaller than 0.074 mm

Rock Continuity	
RECOVERY (%) = $\frac{\text{Total Length of Core} \times 100}{\text{Length of Core Run}}$	
Description	Core Recovery (%)
Incompetent	Less than 40
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Relative Quality of Rocks	
RQD (%) = $\frac{\text{Total core, counting only pieces >4" long}}{\text{Length of Core Run}} \times 100$	
Description	RQD (%)
Very Poor	0-25
Poor	25-50
Fair	50-75
Good	75-90
Excellent	90-100



**Matrix Engineering Group, Inc.**  
engineers | special inspectors | construction consultants

**Correlation of Penetration Resistance with Relative Density and Consistency Sheet and Soil Classification Chart**



# DRILL HOLE LOG

**PROJECT:** Wheeler Park - Playground, Pavillion, and Splash      **PROJECT NO.:** MEG 301287.46  
**CLIENT:** Rockdale County      **DATE:** 3/28/2023  
**LOCATION:** Refer to Figure 1      **ELEVATION:** 883 Feet MSL  
**DRILLER:** Betts      **LOGGED BY:** Sulemana Alhassan  
**DRILLING METHOD:** ASTM D1586 with Automatic Hammer      **STATION:** \_\_\_\_\_  
**DEPTH TO - WATER> INITIAL:**  \_\_\_\_\_ **After 24+ Hours:**  \_\_\_\_\_ **CAVING>**  \_\_\_\_\_

**BORING NO. B-1**

File: Elgranero Borings

Date Printed: 4/7/2023

This information pertains only to this boring and should not be interpreted as being indicative of the site.

ELEVATION (feet)	DEPTH (feet)	Description	SOIL TYPE	SOIL SYMBOL	SAMPLERS	TEST RESULTS					N-Value Blows/ft (ASTM D1586)						
						Natural Moisture Content (%). ▲											
						Penetration - ●											
						10	20	30	40	50							
883	0	Approximately 4 inches Grass and Topsoil Possible Fill - Brown Clayey Sand.	TP Fill														
882	1																
881	2																
880	3																
879	4	Residual - Loose, Brown, Micaceous, Silty SAND with MnO.	SM														
878	5																
877	6																
876	7																
875	8																
874	9																
873	10																
872	11					Boring was Terminated at 10 ft BGS.											
871	12																
870	13																
869	14																
868	15																
867	16																
866	17																
865	18																
864	19																
863	20																
862	21																
861	22																
860	23																
859	24																
858	25																
857	26																
856	27																
855	28																
854	29																

No Groundwater at the time of drilling. Borehole was backfilled at the conclusion of the field work.



# DRILL HOLE LOG

**PROJECT:** Wheeler Park - Playground, Pavillion, and Splash      **PROJECT NO.:** MEG 301287.46  
**CLIENT:** Rockdale County      **DATE:** 3/28/2023  
**LOCATION:** Refer to Figure 1      **ELEVATION:** 891 Feet MSL  
**DRILLER:** Betts      **LOGGED BY:** Sulemana Alhassan  
**DRILLING METHOD:** ASTM D1586 with Automatic Hammer      **STATION:** \_\_\_\_\_  
**DEPTH TO - WATER> INITIAL:**  \_\_\_\_\_ **After 24+ Hours:**  \_\_\_\_\_ **CAVING>**  \_\_\_\_\_

**BORING NO. B-2**

File: Elgranero Borings

Date Printed: 4/7/2023

This information pertains only to this boring and should not be interpreted as being indicative of the site.

ELEVATION (feet)	DEPTH (feet)	Description	SOIL TYPE	SOIL SYMBOL	SAMPLERS	TEST RESULTS					N-Value Blows/ft (ASTM D1586)		
						Natural Moisture Content (%). ▲							
						Penetration - ●							
						10	20	30	40	50			
891	0	Approximately 4 inches Grass and Topsoil Possible Fill - Brown Clayey Sand.	TP Fill										
890	1												
889	2	Residual - Loose, Brown, Micaceous, Silty SAND.	SM										
888	3												
887	4												
886	5												
885	6												
884	7												
883	8												
882	9												
881	10	Boring was Terminated at 10 ft BGS.											
880	11												
879	12												
878	13												
877	14												
876	15												
875	16												
874	17												
873	18												
872	19												
871	20												
870	21												
869	22												
868	23												
867	24												
866	25												
865	26												
864	27												
863	28												
862	29												

No Groundwater at the time of drilling. Borehole was backfilled at the conclusion of the field work.



# DRILL HOLE LOG

**PROJECT:** Wheeler Park - Playground, Pavillion, and Splash      **PROJECT NO.:** MEG 301287.46  
**CLIENT:** Rockdale County      **DATE:** 3/28/2023  
**LOCATION:** Refer to Figure 1      **ELEVATION:** 891 Feet MSL  
**DRILLER:** Betts      **LOGGED BY:** Sulemana Alhassan  
**DRILLING METHOD:** ASTM D1586 with Automatic Hammer      **STATION:** \_\_\_\_\_  
**DEPTH TO - WATER> INITIAL:**  \_\_\_\_\_ **After 24+ Hours:**  \_\_\_\_\_ **CAVING>**  \_\_\_\_\_

**BORING NO. B-3**

File: Elgranero Borings

Date Printed: 4/7/2023

This information pertains only to this boring and should not be interpreted as being indicative of the site.

ELEVATION (feet)	DEPTH (feet)	Description	SOIL TYPE	SOIL SYMBOL	SAMPLERS	TEST RESULTS					N-Value Blows/ft (ASTM D1586)		
						Natural Moisture Content (%). ▲							
						Penetration - ●							
						10	20	30	40	50			
891	0	Approximately 4 inches Grass and Topsoil Possible Fill - Brown Clayey Sand.	TP Fill										
890	1												
889	2												
888	3	Residual - Medium Dense, Micaceous, Silty SAND.	SM										
887	4												
886	5												
885	6												
884	7												
883	8												
882	9												
881	10												
		Boring was Terminated at 10 ft BGS.											
880	11												
879	12												
878	13												
877	14												
876	15												
875	16												
874	17												
873	18												
872	19												
871	20												
870	21												
869	22												
868	23												
867	24												
866	25												
865	26												
864	27												
863	28												
862	29												

No Groundwater at the time of drilling. Borehole was backfilled at the conclusion of the field work.



# DRILL HOLE LOG

**PROJECT:** Wheeler Park - Playground, Pavillion, and Splash      **PROJECT NO.:** MEG 301287.46  
**CLIENT:** Rockdale County      **DATE:** 3/28/2023  
**LOCATION:** Refer to Figure 1      **ELEVATION:** 891 Feet MSL  
**DRILLER:** Betts      **LOGGED BY:** Sulemana Alhassan  
**DRILLING METHOD:** ASTM D1586 with Automatic Hammer      **STATION:**  
**DEPTH TO - WATER> INITIAL:**       **After 24+ Hours:**       **CAVING>**

**BORING NO. B-4**

File: Elgranero Borings

Date Printed: 4/7/2023

This information pertains only to this boring and should not be interpreted as being indicative of the site.

ELEVATION (feet)	DEPTH (feet)	Description	SOIL TYPE	SOIL SYMBOL	SAMPLERS	TEST RESULTS					N-Value Blows/ft (ASTM D1586)		
						Natural Moisture Content (%). ▲							
						Penetration - ●							
						10	20	30	40	50			
891	0	Approximately 4 inches Grass and Topsoil Possible Fill - Brown Silty Sand.	TP Fill										
890	1												
889	2												
888	3	Residual - Medium Dense, Micaceous, Silty SAND.	SM										
887	4												
886	5												
885	6												
884	7												
883	8												
882	9												
881	10												
		Boring was Terminated at 10 ft BGS.											
880	11												
879	12												
878	13												
877	14												
876	15												
875	16												
874	17												
873	18												
872	19												
871	20												
870	21												
869	22												
868	23												
867	24												
866	25												
865	26												
864	27												
863	28												
862	29												

No Groundwater at the time of drilling. Borehole was backfilled at the conclusion of the field work.



# DRILL HOLE LOG

**PROJECT:** Wheeler Park - Playground, Pavillion, and Splash      **PROJECT NO.:** MEG 301287.46  
**CLIENT:** Rockdale County      **DATE:** 3/28/2023  
**LOCATION:** Refer to Figure 1      **ELEVATION:** 891 Feet MSL  
**DRILLER:** Betts      **LOGGED BY:** Sulemana Alhassan  
**DRILLING METHOD:** ASTM D1586 with Automatic Hammer      **STATION:** \_\_\_\_\_  
**DEPTH TO - WATER> INITIAL:**  \_\_\_\_\_ **After 24+ Hours:**  \_\_\_\_\_ **CAVING>**  \_\_\_\_\_

**BORING NO. B-5**

File: Elgranero Borings

Date Printed: 4/7/2023

This information pertains only to this boring and should not be interpreted as being indicative of the site.

ELEVATION (feet)	DEPTH (feet)	Description	SOIL TYPE	SOIL SYMBOL	SAMPLERS	TEST RESULTS					N-Value Blows/ft (ASTM D1586)			
						Natural Moisture Content (%). ▲ Penetration - ●								
						10	20	30	40	50				
891	0	Approximately 4 inches Grass and Topsoil Possible Fill - Brown Silty Sand.	TP Fill											
890	1													
889	2													
888	3													
887	4	Medium Dense, Brown, Clayey SAND.	SC											
886	5													
885	6													
884	7													
883	8													
882	9													
881	10													
881	10	Boring was Terminated at 10 ft BGS.												
880	11													
879	12													
878	13													
877	14													
876	15													
875	16													
874	17													
873	18													
872	19													
871	20													
870	21													
869	22													
868	23													
867	24													
866	25													
865	26													
864	27													
863	28													
862	29													

No Groundwater at the time of drilling. Borehole was backfilled at the conclusion of the field work.



# DRILL HOLE LOG

**PROJECT:** Wheeler Park - Playground, Pavillion, and Splash      **PROJECT NO.:** MEG 301287.46  
**CLIENT:** Rockdale County      **DATE:** 3/28/2023  
**LOCATION:** Refer to Figure 1      **ELEVATION:** 891 Feet MSL  
**DRILLER:** Betts      **LOGGED BY:** Sulemana Alhassan  
**DRILLING METHOD:** ASTM D1586 with Automatic Hammer      **STATION:** \_\_\_\_\_  
**DEPTH TO - WATER> INITIAL:**  \_\_\_\_\_ **After 24+ Hours:**  \_\_\_\_\_ **CAVING>**  \_\_\_\_\_

**BORING NO. B-6**

File: Elgranero Borings

Date Printed: 4/7/2023

This information pertains only to this boring and should not be interpreted as being indicative of the site.

ELEVATION (feet)	DEPTH (feet)	Description	SOIL TYPE	SOIL SYMBOL	SAMPLERS	TEST RESULTS					N-Value Blows/ft (ASTM D1586)		
						Natural Moisture Content (%). ▲							
						Penetration - ●							
						10	20	30	40	50			
891	0	Approximately 4 inches Grass and Topsoil Possible Fill - Brown Silty Sand.	TP Fill										
890	1												
889	2												
888	3	Residual - Medium Dense, Micaceous, Silty SAND.	SM										
887	4												
886	5												
885	6												
884	7												
883	8												
882	9												
881	10												
		Boring was Terminated at 10 ft BGS.											
880	11												
879	12												
878	13												
877	14												
876	15												
875	16												
874	17												
873	18												
872	19												
871	20												
870	21												
869	22												
868	23												
867	24												
866	25												
865	26												
864	27												
863	28												
862	29												

No Groundwater at the time of drilling. Borehole was backfilled at the conclusion of the field work.





# DRILL HOLE LOG

**PROJECT:** Wheeler Park - Playground, Pavillion, and Splash      **PROJECT NO.:** MEG 301287.46  
**CLIENT:** Rockdale County      **DATE:** 3/28/2023  
**LOCATION:** Refer to Figure 1      **ELEVATION:** 891 Feet MSL  
**DRILLER:** Betts      **LOGGED BY:** Sulemana Alhassan  
**DRILLING METHOD:** ASTM D1586 with Automatic Hammer      **STATION:**  
**DEPTH TO - WATER> INITIAL:**       **After 24+ Hours:**       **CAVING>**

**BORING NO. B-7**

File: Elgranero Borings

Date Printed: 4/7/2023

This information pertains only to this boring and should not be interpreted as being indicative of the site.

ELEVATION (feet)	DEPTH (feet)	Description	SOIL TYPE	SOIL SYMBOL	SAMPLERS	TEST RESULTS					N-Value Blows/ft (ASTM D1586)		
						Natural Moisture Content (%). ▲							
						Penetration - ●							
						10	20	30	40	50			
891	0	Approximately 4 inches Grass and Topsoil Possible Fill - Brown Silty Sand.	TP Fill										
890	1												
889	2												
888	3	Residual - Loose, Light Brown, Micaceous, Silty SAND.	SM										
887	4												
886	5												
885	6												
884	7												
883	8												
882	9												
881	10												
881	10	Boring was Terminated at 10 ft BGS.											
880	11												
879	12												
878	13												
877	14												
876	15												
875	16												
874	17												
873	18												
872	19												
871	20												
870	21												
869	22												
868	23												
867	24												
866	25												
865	26												
864	27												
863	28												
862	29												

No Groundwater at the time of drilling. Borehole was backfilled at the conclusion of the field work.



# DRILL HOLE LOG

**PROJECT:** Wheeler Park - Playground, Pavillion, and Splash      **PROJECT NO.:** MEG 301287.46  
**CLIENT:** Rockdale County      **DATE:** 3/28/2023  
**LOCATION:** Refer to Figure 1      **ELEVATION:** 883 Feet MSL  
**DRILLER:** Betts      **LOGGED BY:** Sulemana Alhassan  
**DRILLING METHOD:** ASTM D1586 with Automatic Hammer      **STATION:** \_\_\_\_\_  
**DEPTH TO - WATER> INITIAL:**  \_\_\_\_\_ **After 24+ Hours:**  \_\_\_\_\_ **CAVING>**  \_\_\_\_\_

**BORING NO. B-8**

File: Elgranero Borings

Date Printed: 4/7/2023

This information pertains only to this boring and should not be interpreted as being indicative of the site.

ELEVATION (feet)	DEPTH (feet)	Description	SOIL TYPE	SOIL SYMBOL	SAMPLERS	TEST RESULTS					N-Value Blows/ft (ASTM D1586)	
						Natural Moisture Content (%). ▲						
						Penetration - ●						
						10	20	30	40	50		
883	0	Approximately 4 inches Grass and Topsoil Possible Fill - Brown Silty Sand.	TP Fill									
882	1											
881	2											
880	3											
879	4	Medium Dense, Brown, Clayey SAND.	SC									
878	5											
877	6											
876	7											
875	8											
874	9											
873	10											
872	11	Boring was Terminated at 10 ft BGS.										
871	12											
870	13											
869	14											
868	15											
867	16											
866	17											
865	18											
864	19											
863	20											
862	21											
861	22											
860	23											
859	24											
858	25											
857	26											
856	27											
855	28											
854	29											

No Groundwater at the time of drilling. Borehole was backfilled at the conclusion of the field work.



# DRILL HOLE LOG

**PROJECT:** Wheeler Park - Playground, Pavillion, and Splash      **PROJECT NO.:** MEG 301287.46  
**CLIENT:** Rockdale County      **DATE:** 3/28/2023  
**LOCATION:** Refer to Figure 1      **ELEVATION:** 885 Feet MSL  
**DRILLER:** Betts      **LOGGED BY:** Sulemana Alhassan  
**DRILLING METHOD:** ASTM D1586 with Automatic Hammer      **STATION:** \_\_\_\_\_  
**DEPTH TO - WATER> INITIAL:**  \_\_\_\_\_ **After 24+ Hours:**  \_\_\_\_\_ **CAVING>**  \_\_\_\_\_

**BORING NO. B-9**

File: Elgranero Borings

Date Printed: 4/7/2023

This information pertains only to this boring and should not be interpreted as being indicative of the site.

ELEVATION (feet)	DEPTH (feet)	Description	SOIL TYPE	SOIL SYMBOL	SAMPLERS	TEST RESULTS					N-Value Blows/ft (ASTM D1586)		
						Natural Moisture Content (%). ▲							
						Penetration - ●							
						10	20	30	40	50			
885	0	Approximately 4 inches Grass and Topsoil Possible Fill - Brown Silty Sand.	TP Fill										
884	1												
883	2												
882	3	Residual - Medium Dense, Micaceous, Clayey SAND.	SC										
881	4												
880	5												
879	6												
878	7												
877	8												
876	9												
875	10												
		Boring was Terminated at 10 ft BGS.											
874	11												
873	12												
872	13												
871	14												
870	15												
869	16												
868	17												
867	18												
866	19												
865	20												
864	21												
863	22												
862	23												
861	24												
860	25												
859	26												
858	27												
857	28												
856	29												

No Groundwater at the time of drilling. Borehole was backfilled at the conclusion of the field work.



# DRILL HOLE LOG

**PROJECT:** Wheeler Park - Playground, Pavillion, and Splash      **PROJECT NO.:** MEG 301287.46  
**CLIENT:** Rockdale County      **DATE:** 3/28/2023  
**LOCATION:** Refer to Figure 1      **ELEVATION:** 885 Feet MSL  
**DRILLER:** Betts      **LOGGED BY:** Sulemana Alhassan  
**DRILLING METHOD:** ASTM D1586 with Automatic Hammer      **STATION:** \_\_\_\_\_  
**DEPTH TO - WATER> INITIAL:**  \_\_\_\_\_ **After 24+ Hours:**  \_\_\_\_\_ **CAVING>**  \_\_\_\_\_

**BORING NO. B-10**

File: Elgranero Borings

Date Printed: 4/7/2023

This information pertains only to this boring and should not be interpreted as being indicative of the site.

ELEVATION (feet)	DEPTH (feet)	Description	SOIL TYPE	SOIL SYMBOL	SAMPLERS	TEST RESULTS					N-Value Blows/ft (ASTM D1586)
						Natural Moisture Content (%). ▲					
885	0	Approximately 4 inches Grass and Topsoil Possible Fill - Brown Clayey Sand.	TP Fill			Penetration - ●					11
884	1					10 20 30 40 50					
883	2	Medium Dense, Dark Brown, Clayey SAND.	SC								
882	3										
881	4										
880	5										
879	6										
878	7										
877	8										
876	9										
875	10										
874	11	Boring was Terminated at 10 ft BGS.									
873	12										
872	13										
871	14										
870	15										
869	16										
868	17										
867	18										
866	19										
865	20										
864	21										
863	22										
862	23										
861	24										
860	25										
859	26										
858	27										
857	28										
856	29										

No Groundwater at the time of drilling. Borehole was backfilled at the conclusion of the field work.



# DRILL HOLE LOG

**PROJECT:** Wheeler Park - Playground, Pavillion, and Splash      **PROJECT NO.:** MEG 301287.46  
**CLIENT:** Rockdale County      **DATE:** 3/28/2023  
**LOCATION:** Refer to Figure 1      **ELEVATION:** 882 Feet MSL  
**DRILLER:** Betts      **LOGGED BY:** Sulemana Alhassan  
**DRILLING METHOD:** ASTM D1586 with Automatic Hammer      **STATION:** \_\_\_\_\_  
**DEPTH TO - WATER> INITIAL:**  \_\_\_\_\_ **After 24+ Hours:**  \_\_\_\_\_ **CAVING>**  \_\_\_\_\_

**BORING NO. B-11**

File: Elgranero Borings

Date Printed: 4/7/2023

This information pertains only to this boring and should not be interpreted as being indicative of the site.

ELEVATION (feet)	DEPTH (feet)	Description	SOIL TYPE	SOIL SYMBOL	SAMPLERS	TEST RESULTS					N-Value Blows/ft (ASTM D1586)			
						Natural Moisture Content (%). ▲								
						Penetration - ●								
						10	20	30	40	50				
882	0	Approximately 4 inches Grass and Topsoil Possible Fill - Brown Clayey Sand.	TP Fill											
881	1													
880	2													
879	3	Medium Dense, Dark Brown, Clayey SAND.	SC											
878	4													
877	5													
876	6													
875	7													
874	8													
873	9													
872	10													
871	11	Boring was Terminated at 10 ft BGS.												
870	12													
869	13													
868	14													
867	15													
866	16													
865	17													
864	18													
863	19													
862	20													
861	21													
860	22													
859	23													
858	24													
857	25													
856	26													
855	27													
854	28													
853	29													

No Groundwater at the time of drilling. Borehole was backfilled at the conclusion of the field work.



# DRILL HOLE LOG

**PROJECT:** Wheeler Park - Playground, Pavillion, and Splash      **PROJECT NO.:** MEG 301287.46  
**CLIENT:** Rockdale County      **DATE:** 3/28/2023  
**LOCATION:** Refer to Figure 1      **ELEVATION:** 882 Feet MSL  
**DRILLER:** Betts      **LOGGED BY:** Sulemana Alhassan  
**DRILLING METHOD:** ASTM D1586 with Automatic Hammer      **STATION:** \_\_\_\_\_  
**DEPTH TO - WATER> INITIAL:**  \_\_\_\_\_ **After 24+ Hours:**  \_\_\_\_\_ **CAVING>**  \_\_\_\_\_

**BORING NO. B-12**

File: Elgranero Borings

Date Printed: 4/7/2023

This information pertains only to this boring and should not be interpreted as being indicative of the site.

ELEVATION (feet)	DEPTH (feet)	Description	SOIL TYPE	SOIL SYMBOL	SAMPLERS	TEST RESULTS					N-Value Blows/ft (ASTM D1586)		
						Natural Moisture Content (%). ▲							
						Penetration - ●							
						10	20	30	40	50			
882	0	Approximately 4 inches Grass and Topsoil Possible Fill - Yellowish Brown Clayey Sand.	TP Fill										
881	1												
880	2												
879	3	Residual - Medium Dense, Yellowish Brown, Micaceous, Silty SAND.	SM										
878	4												
877	5												
876	6												
875	7												
874	8												
873	9												
872	10												
		Boring was Terminated at 10 ft BGS.											
871	11												
870	12												
869	13												
868	14												
867	15												
866	16												
865	17												
864	18												
863	19												
862	20												
861	21												
860	22												
859	23												
858	24												
857	25												
856	26												
855	27												
854	28												
853	29												

No Groundwater at the time of drilling. Borehole was backfilled at the conclusion of the field work.



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Project Name: Wheeler Park - Playground, Pavilion and Splashpad

Project Location: Wheeler Park, 1400 Parker Rd SE, Conyers, GA 30094, USA

Client: Rockdale County

Project Code: MEG 301287.46

Preparer: Sulemana Alhassan

Reviewer: Sam Alyateem, PE

Report Date: 04/07/2023

Page Number: 1 of 8



Picture 1: B-1



Picture 2: B-3

Latitude: 33.6506  
 Longitude: -84.0192  
 Bearing: N

Weather: Cloudy  
 Date Taken: 03/28/2023  
 Taken By: Sulemana J. Alhassan

Latitude: 33.6511  
 Longitude: -84.0189  
 Bearing: SW

Weather: Partly cloudy  
 Date Taken: 03/28/2023  
 Taken By: Sulemana J. Alhassan

Tags:

Tags:



Project Name: Wheeler Park - Playground, Pavilion and Splashpad

Project Location: Wheeler Park, 1400 Parker Rd SE, Conyers, GA 30094, USA

Client: Rockdale County

Project Code: MEG 301287.46

Preparer: Sulemana Alhassan

Reviewer: Sam Alyateem, PE

Report Date: 04/07/2023

Page Number: 2 of 8





Picture 3: B-4



Picture 4: B-5

Latitude: 33.6509

Longitude: -84.0191

Bearing: N

Weather: Partly cloudy

Date Taken: 03/28/2023

Taken By: Sulemana J. Alhassan

Latitude: 33.6511

Longitude: -84.0189

Bearing: W

Weather: Partly cloudy

Date Taken: 03/28/2023

Taken By: Sulemana J. Alhassan

Tags:

Tags:



Project Name: Wheeler Park - Playground, Pavilion and Splashpad

Project Location: Wheeler Park, 1400 Parker Rd SE, Conyers, GA 30094, USA

Client: Rockdale County

Project Code: MEG 301287.46

Preparer: Sulemana Alhassan

Reviewer: Sam Alyateem, PE

Report Date: 04/07/2023

Page Number: 3 of 8



Picture 5: B-6



Picture 6: B-7

Latitude: 33.6510  
 Longitude: -84.0188  
 Bearing: E

Weather: Partly cloudy  
 Date Taken: 03/28/2023  
 Taken By: Sulemana J. Alhassan

Latitude: 33.6509  
 Longitude: -84.0187  
 Bearing: NE

Weather: Partly cloudy  
 Date Taken: 03/28/2023  
 Taken By: Sulemana J. Alhassan

Tags:

Tags:



Project Name: Wheeler Park - Playground, Pavilion and Splashpad

Project Location: Wheeler Park, 1400 Parker Rd SE, Conyers, GA 30094, USA

Client: Rockdale County

Project Code: MEG 301287.46

Preparer: Sulemana Alhassan

Reviewer: Sam Alyateem, PE

Report Date: 04/07/2023

Page Number: 4 of 8



Picture 7: B-8



Picture 8: B-9

Latitude: 33.6506  
 Longitude: -84.0187  
 Bearing: N

Weather: Partly cloudy  
 Date Taken: 03/28/2023  
 Taken By: Sulemana J. Alhassan

Latitude: 33.6507  
 Longitude: -84.0189  
 Bearing: E

Weather: Partly cloudy  
 Date Taken: 03/28/2023  
 Taken By: Sulemana J. Alhassan

Tags:

Tags:



Project Name: Wheeler Park - Playground, Pavilion and Splashpad

Project Location: Wheeler Park, 1400 Parker Rd SE, Conyers, GA 30094, USA

Client: Rockdale County

Project Code: MEG 301287.46

Preparer: Sulemana Alhassan

Reviewer: Sam Alyateem, PE

Report Date: 04/07/2023

Page Number: 5 of 8



Picture 9: B-10



Picture 10: B-11

Latitude: 33.6506  
 Longitude: -84.0186  
 Bearing: NE

Weather: Sunny  
 Date Taken: 03/28/2023  
 Taken By: Sulemana J. Alhassan

Latitude: 33.6506  
 Longitude: -84.0187  
 Bearing: NE

Weather: Sunny  
 Date Taken: 03/28/2023  
 Taken By: Sulemana J. Alhassan

Tags:

Tags:



Project Name: Wheeler Park - Playground, Pavilion and Splashpad

Project Location: Wheeler Park, 1400 Parker Rd SE, Conyers, GA 30094, USA

Client: Rockdale County

Project Code: MEG 301287.46

Preparer: Sulemana Alhassan

Reviewer: Sam Alyateem, PE

Report Date: 04/07/2023

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Picture 11

Picture 12

Latitude: 33.6506  
 Longitude: -84.0194  
 Bearing: NE

Weather: Cloudy  
 Date Taken: 03/28/2023  
 Taken By: Sulemana J. Alhassan

Latitude: 33.6506  
 Longitude: -84.0195  
 Bearing: W

Weather: Cloudy  
 Date Taken: 03/28/2023  
 Taken By: Sulemana J. Alhassan

Tags:

Tags:



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Picture 13

Picture 14

Latitude: 33.6506  
 Longitude: -84.0194  
 Bearing: NE

Weather: Cloudy  
 Date Taken: 03/28/2023  
 Taken By: Sulemana J. Alhassan

Latitude: 33.6515  
 Longitude: -84.0184  
 Bearing: SW

Weather: Partly cloudy  
 Date Taken: 03/28/2023  
 Taken By: Sulemana J. Alhassan

Tags:

Tags:



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Picture 1: B-1

Picture 2: B-3

Latitude: 33.6506  
 Longitude: -84.0192  
 Bearing: N

Weather: Cloudy  
 Date Taken: 03/28/2023  
 Taken By: Sulemana J. Alhassan

Latitude: 33.6511  
 Longitude: -84.0189  
 Bearing: SW

Weather: Partly cloudy  
 Date Taken: 03/28/2023  
 Taken By: Sulemana J. Alhassan

Tags:

Tags:



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Picture 3: B-4



Picture 4: B-5

Latitude: 33.6509

Longitude: -84.0191

Bearing: N

Weather: Partly cloudy

Date Taken: 03/28/2023

Taken By: Sulemana J. Alhassan

Latitude: 33.6511

Longitude: -84.0189

Bearing: W

Weather: Partly cloudy

Date Taken: 03/28/2023

Taken By: Sulemana J. Alhassan

Tags:

Tags:



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Picture 5: B-6



Picture 6: B-7

Latitude: 33.6510  
 Longitude: -84.0188  
 Bearing: E

Weather: Partly cloudy  
 Date Taken: 03/28/2023  
 Taken By: Sulemana J. Alhassan

Latitude: 33.6509  
 Longitude: -84.0187  
 Bearing: NE

Weather: Partly cloudy  
 Date Taken: 03/28/2023  
 Taken By: Sulemana J. Alhassan

Tags:

Tags:



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Picture 7: B-8



Picture 8: B-9

Latitude: 33.6506  
 Longitude: -84.0187  
 Bearing: N

Weather: Partly cloudy  
 Date Taken: 03/28/2023  
 Taken By: Sulemana J. Alhassan

Latitude: 33.6507  
 Longitude: -84.0189  
 Bearing: E

Weather: Partly cloudy  
 Date Taken: 03/28/2023  
 Taken By: Sulemana J. Alhassan

Tags:

Tags:



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Picture 9: B-10



Picture 10: B-11

Latitude: 33.6506  
 Longitude: -84.0186  
 Bearing: NE

Weather: Sunny  
 Date Taken: 03/28/2023  
 Taken By: Sulemana J. Alhassan

Latitude: 33.6506  
 Longitude: -84.0187  
 Bearing: NE

Weather: Sunny  
 Date Taken: 03/28/2023  
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Tags:

Tags:



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Latitude: 33.6506  
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 Bearing: W

Weather: Cloudy  
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